

ADVANTAGE STEEL

NO. 58 SUMMER 2017

BRIDGE ISSUE



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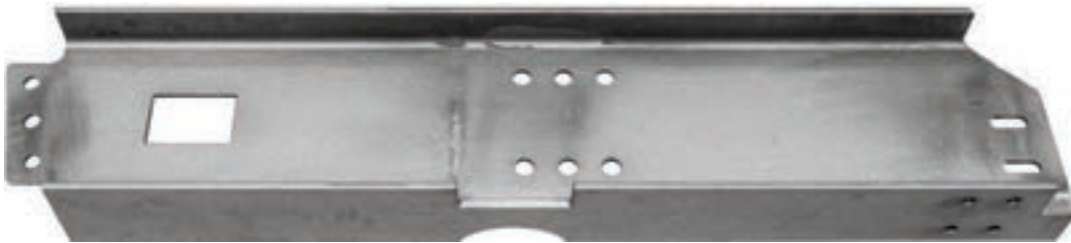
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38



14



28



FEATURES

- 14 Bridging the Remote North
Fort Nelson River Bridge gets a vital makeover
Raj Singh, P.Eng., PE; C.P. (Ken) Rebel, P.Eng.; Chad Amiel, P.Eng.
- 22 A Gateway to Renewal
The John Street Pedestrian Bridge joins two worlds
Aaron Bean, Project Manager, Walters Inc.
- 28 Flyover Bridges
NE Anthony Henday Drive and Yellowhead Trail Interchange
Myles Eugene Henry Lewis, P.Eng., Structural Engineer
- 38 An Essential Link
The design and construction of the Sir Ambrose Shea vertical lift bridge
Hellen Christodoulou, Ph.D.ing., B.C.L., LL.B., M.B.A., CISC Quebec Region Manager

IN EVERY ISSUE

- 4 From the President
Ed Whalen, P.Eng.
- 52 News and Events
- 61 Member and Associate Products / Services Directory

COLUMNS

- 8 Technical Column
Alfred F. Wong, P.Eng.
- 10 Seismic Corner
Evolution in Seismic Design of Steel Buildings
Alfred F. Wong, P.Eng.
- 12 For Green's Sake
Tareq Ali, RPM

22



Professional engineers, architects, structural steel fabricators and others interested in steel construction are invited to inquire about CISC membership. Readers are encouraged to submit their interesting steel construction projects for consideration for inclusion in this publication by contacting CISC.



On the Cover:
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“FISC” My new favourite acronym

IN JANUARY, I WROTE about the CISC led trade case launched in September 2016 against dumped and subsidized industrial structural steel and platework aka FISC originating from China, South Korea and Spain. The case is now over, the dust settled and the findings and reasons published.

The acronym “FISC” stands for fabricated industrial steel components. A key and important word is “industrial”. We focused our case only on industrial and within specific industrial sectors. In the context of our trade case, FISC includes: steel beams, columns, braces, frames, railings, stairs, trusses, conveyor belt frame structures and galleries, bents, bins, chutes, hoppers, ductwork, process tanks, pipe racks and apron feeders, whether assembled or partially assembled into modules, or unassembled, for use in industrial structures for:

1. Buildings
2. Process equipment, process enclosures
3. Access structures and process structures
4. Structures for conveyancing and material handling in the following sectors:
 - Oil and gas extraction, conveyance and processing
 - Mining extraction, conveyance, storage, and processing
 - Industrial power generation facilities
 - Petrochemical plants
 - Cement plants
 - Fertilizer plants
 - Industrial metal smelters

As you will notice it includes fabrication of stick steel, full & partial assemblies and frames right up to the modern module of all sizes and complexity. There are a few more details to what’s listed above and I encourage you to visit our website for full details.

Opposition to our case came surprisingly from within Canada and focused primarily on modules, arguing that modules are not structures but are a manufactured product. Tell that to the building official!

What was interesting and eye opening was the focus by opponents on BC. This is a heads up to where the

next battleground lies. They asked the Tribunal to exclude modules in BC stating that due to projects being along the coast, the modules could be made extremely large and complex (full of equipment and other trades’ products (plug and play)), and due to lower cost in countries such as China, would be procured outside of Canada no matter what. So, for those in other non-FISC industrial construction sectors, if you are hoping future LNG construction projects (if they ever get the provincial nod) will help the BC economy and your company, think again. If no action is taken by your industry now you are most likely not to get any work at all from these projects if they have their way.

Modularization is fast becoming the construction method of choice for industrial construction around the world putting construction assembly in modular yards rather than at the project site where labour, safety and productivity are sometimes a factor. Modularization can also be used to take advantage of illegal artificially low prices from dumping and subsidizing countries. The ultimate module is the largest that one can make and transport. It includes everything including FISC, piping, electrical, mechanical equipment, instrumentation, siding, roofing, etc. For coastal projects, it not uncommon now to have complex modules in the 1000-8000+ tonnes range. Make these modules in a country that is known for its dumping and subsidizing practices and there is no country or industrial construction sector in the world that will be able to compete. Countries will lose not only their steel industries but also all the other construction trades and industries feeding into these projects.

So, what was the outcome of our trade case?

1. Canada Border Services Agency (CBSA) proved that China, South Korea and Spain were illegally dumping into Canada. China was also found to be illegally subsidizing their industry. There were some companies that had lower dumping margins but in general the range of dumping was in the 42% to 46%. In the case of China, they were also assessed with an average subsidizing margin of 70%.

	<p>CHAIRMAN Laurier Trudeau, Abesco Ltd.</p>	<p>MANAGING EDITOR Tareq Ali, CISC</p>
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2. The Canadian International Trade Tribunal (CITT) found in favour of our claim of harm, implementing a tariff on the subject countries effective January 2017 for 5 years.
3. The CITT ruled against requests for exclusions of modules of FISC and non-FISC components (think mechanical, electrical, piping etc.) leaving the decision of what is applicable to CBSA on a case-by-case basis.
4. The actual tariffs applied will be calculated on a project-by-project basis using normalized values by CBSA. The importer is the party required to pay the tariff.
5. The tariff applied is payable to the Government of Canada.
6. The steel construction industry does not receive any of the tariff funds collected by the government.

What did we prove? We proved our assertion that the subject countries were indeed illegally dumping into our country. We proved illegal dumping and subsidizing and not global competitiveness were the reasons for the dramatic difference in pricing. We proved that the Canadian steel construction industry will stand up and fight for trade fairness for our industry, for our employees, their families and for Canada. This ruling allows Canadian companies and their supply chains to benefit in Canadian projects, and compete on a level and fair playing field. That is all they want. They are not looking for protection but demanding fairness. Canadian companies must abide by the rules and therefore foreign companies playing in our markets must be held to the same standard.

The CISC has paved the way for other Canadian industries of complex products to protect themselves against dumping and subsidizing. It is a tough and hard road with many uncertainties but if faced with a similar situation, as we did with FISC, you have no choice. **AS**

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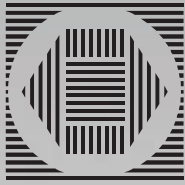
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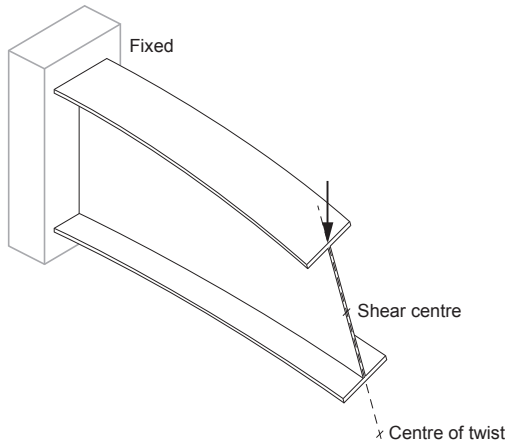


Alfred F. Wong, P.Eng., F.CSCE
Director of Engineering

CISC provides this column as a part of its commitment to the education of those interested in the use of steel in construction. Neither CISC nor the author assumes responsibility for errors or oversights resulting from the use of the information contained herein. Suggested solutions may not necessarily apply to a particular structure or application, and are not intended to replace the expertise of a professional engineer, architect or other licensed professional.

Question 1: In the design of a cantilever wide-flange beam with a fully fixed end and subjected to a point load at the tip, what is the most effective braced point against lateral-torsional buckling? I can provide a lateral support near the tip, either to the compression flange or the tension flange.

Answer: Contrary to common misconception, lateral support to the tension flange at the cantilever tip is more effective. In other words, a tension-flange braced cantilever has a larger lateral-torsional buckling resistance. As shown in the Figure, the twist centre of the buckled shape is on the compression side of the neutral axis.



ERRATUM. In *Advantage Steel* #44, this column referenced the expressions for the elastic lateral-torsional buckling moment of cantilevers provided in the *Guide to Stability Design Criteria for Metal Structures, 6th Edition*. In comparison with recent studies using finite element analyses, the expression " $M_c = 1.5GJ/d$ " gives unconservative values for plates (rectangular section) and long cantilevers of I-sections prone to lateral-torsional buckling. It should not be used for plate cantilevers significantly longer than twice their depth.

Question 2: I have another question: Does the load application point in relation to the shear centre affect the buckling resistance?

Answer: Yes, unless the cantilever tip is fully braced against rotation, e.g. both flanges are laterally supported. When only one flange is laterally supported,

a load applied on the tension side of the shear centre creates a destabilizing effect, resulting in a smaller buckling resistance.

Question 3: After decades of professional practice, I recently heard that limit states design of steel beams and joists that require sprayed fire protection should include 'load restriction factors'. Should I?

Answer: UL Directories provide comprehensive cUL fire-rated designs that are applicable in Canada and are free from load restriction. These cUL Designs for floor assemblies, beams and open-web steel joists, together with several non-load-restricted ULC Designs, capture all common applications in building construction. Part 1 of the recently launched *CISC Steel Design Series "ULC and cUL Sprayed-Applied Fire-Rated Designs"* includes a summary of these non-load-restricted Designs.

Question 4: Are 8-millimetre-thick hardened washers required for pretensioned large A490 bolts used in oversized and slotted holes?

Answer: In the recently issued Update to CSA S16-14, Clause 23.4.2 d) specifies 8-millimetre-thick ASTM F436 hardened washers for pretensioned A490 bolts greater than 26 mm used in oversized and slotted holes, except that F436 washers in combination with a 10-mm plate washer covering the holes may be used.

Question 5: The CISC Handbook tabulates C_w -values for open sections but excludes them for HSS. Why?

Answer: HSS are closed sections and possess much larger St. Venant torsional stiffness as compared to their warping counterpart. C_w may be conservatively ignored for practical purposes. **AS**

Questions on various aspects of design and construction of steel buildings and bridges are welcome. They may be submitted via email to info@cisc-icca.ca. CISC receives and attends to a large volume of inquiries; only a selected few are published in this column.

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Alfred F. Wong, P.Eng., F.CSCE
Director of Engineering

Evolution in Seismic Design of Steel Buildings

MAINSTREAM SEISMIC design requirements for building structures have evolved since the introduction of the National Building Code. In its first edition, NBC 1941 (see Figure 1), buildings in locations where destructive earthquakes were probable were required to resist a static horizontal force acting in any possible direction at its centre of gravity. The static force was

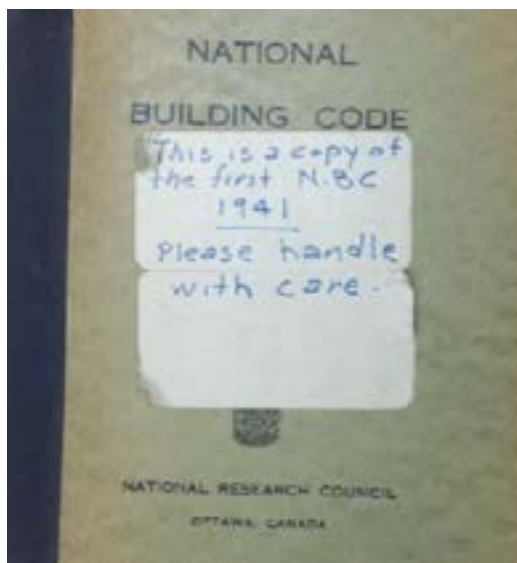


FIGURE 1:

given as a function of the building weight and a constant C , where the value of C depended on whether the allowable bearing value of the soil was greater than one ton per square foot. Yes, that was it!

Development of seismic maps and zones, recognition and incorporation of dynamic properties including natural period(s) and damping and other influential parameters such as accidental torsion, followed. In NBCC 1965, seismic force-resisting systems (SFRS) were divided into two groups in recognition of the differences in performance and ductile behaviour (or lack thereof). Accordingly, building types that featured moment-resisting frames and ductile shear walls were assigned a force multiplier, $K = 0.7$, whereas other types of building were assigned $K = 1.25$. Importance factors were also introduced. Subsequently, building types and their associated K -factors were expanded. In NBCC 1975,



FIGURE 2:

there were five building types, having K -values from 0.7 for moment frame buildings to 2.0 for unreinforced masonry buildings. Generally, these K -values were established based on observation of past performance and ranking in relation to each other.

The revolution in seismic design of steel framed buildings began with the introduction of NBC 1990 and CSA Standard S16.1-M89. Developments up to the current S16-14 edition, contained in the 5th through the 11th edition of the CISC Handbook of Steel Construction (see Figure 2), are summarized below.

CSA S16.1-M89

In a very comprehensive new clause, Clause 27, S16.1-M89 introduced specific design and detailing requirements for most steel SFRS referenced in NBC 1990. Force modification factors R , which replaced the K -factors, applied as divisors. For a system that qualified for an $R > 1.5$, the design was required to meet a specific set of requirements in Clause 27 of S16.1-M89.

Clause 27 covered ductile design requirements for five SFRS:

- Ductile moment-resisting frames
- Moment-resisting frames with nominal ductility
- Ductile braced frames
- Braced frames with nominal ductility, and
- Eccentrically braced frames

Since the 1989 edition of S16.1, connections in ductile or nominally ductile braced frames have been required to resist the gross-section tensile yielding capacity of the diagonal brace member, $A_g F_y$. An historical overview of this requirement can be found in Advantage Steel No. 23.

CSA S16.1-94

Seismic requirements in Clause 27 were somewhat relaxed for structures in low seismic zones. Seismic design of steel-plate shear wall was introduced.

CSA S16-01

Moderately-ductile moment-resisting frames were added to the list of SFRS in S16-01. Ductile concentrically braced frames were replaced with moderately-ductile concentrically braced

frames. Although implicit in previous editions of S16, Conventional Construction ($R = 1.5$) was formally added to Clause 27 along with specific requirements.

The design of moment connections in ductile and moderately-ductile moment-resisting frames must be based on physical testing of full scale connections.

In S16-01 Supplement No. 1 (issued in 2005), echoed a change incorporated in NBC 2005, replacing the force modification factor, R , with the product $R_d R_o$, where R_d and R_o are ductility-related and overstrength-related force modification factors, respectively.

CSA S16-09

The evolution of seismic design continued in 2009 with the addition of new SFRS such

as limited-ductility plate walls and ductile buckling-restrained brace frames. Height limits for Conventional Construction in moderate and high seismicities were extended, provided a variety of conditions were satisfied, and the concept of "protected zone" was introduced.

CSA S16-14

Material requirements for SFRS were expanded in 2014 with the introduction of "demand-critical welds", which are discussed in Advantage Steel No. 56. Built-up and modular link beams offered new advantages for buildings incorporating eccentrically braced frames.

Many changes have been incorporated in the NBC during the last two decades. Some of them have also affected the seismic design of steel-framed buildings. **AS**

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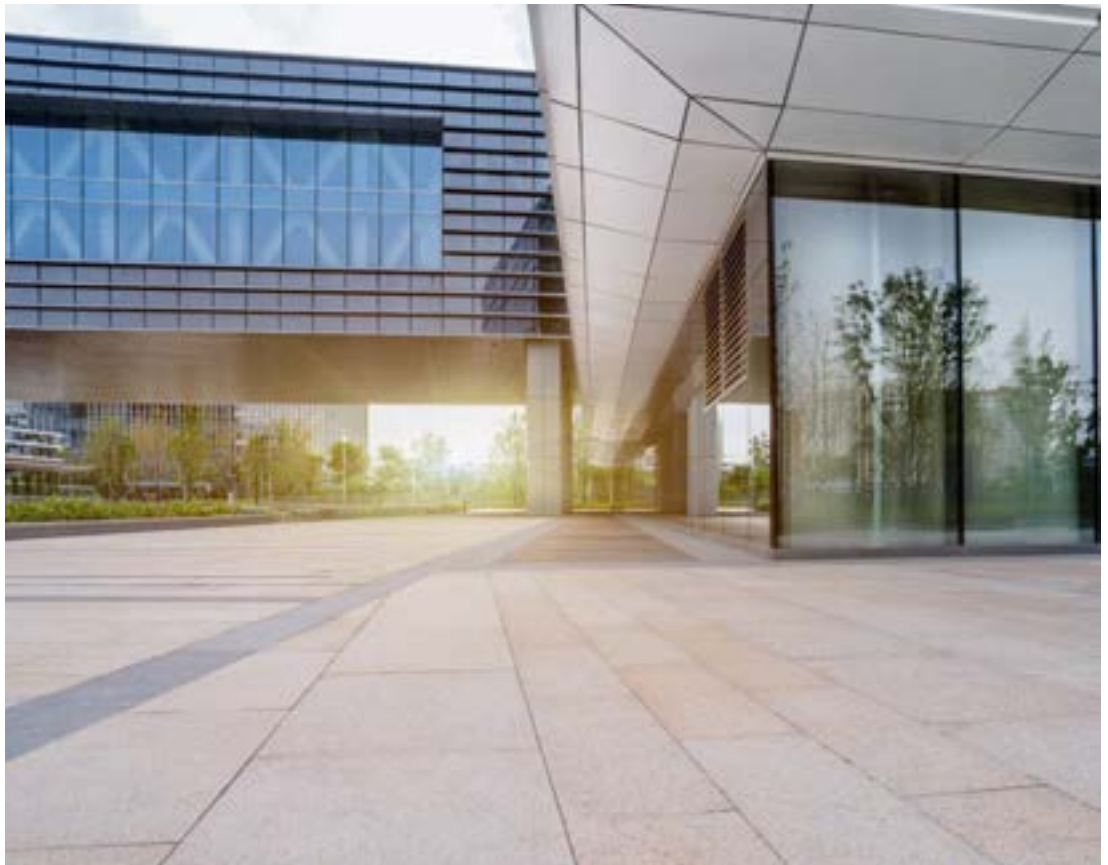


Tareq Ali, RPM
**Director of Marketing
and Communications**

The Zero Carbon Building Standard

A bold effort to accelerate the transition to zero carbon building construction

The Canadian building industry now has a robust new tool to help them construct highly energy-efficient, zero carbon buildings in the future with the launch of the Canada Green Building Council (CaGBC)'s ground breaking Zero Carbon Building Standard.



THIS UNIQUE, CANADIAN CRAFTED Standard assesses carbon use in commercial, institutional, and multi-family buildings in Canada, providing a path for both new and existing buildings to reach zero carbon. A zero carbon building is defined as one that is highly energy-efficient and produces onsite, or procures, carbon-free renewable energy in an amount sufficient to offset the annual carbon emissions associated with its operations.

The Standard's focus on carbon is crucial, as the most important factor in the carbon footprint of a building is often not energy performance, but the carbon intensity of the local electrical grid and the fossil fuels used. Recognizing these differences is therefore critical to

accurately assessing impacts and guiding investments in energy efficiency, heating technologies, and renewable energy.

The Zero Carbon Building Standard is part of a larger CaGBC Zero Carbon Building Initiative that was created to champion the move to lower-carbon commercial, institutional and high-rise residential buildings in support of Canada's efforts to reduce greenhouse gas emissions by 30 per cent by 2030.

It was developed by CaGBC and Integral Group, in extensive consultation with representatives from over 50 industry organizations, utilities, governments and companies across Canada.

Key components of the Zero Carbon Building Standard include:

- 1. Zero Carbon Balance:** No net greenhouse gas (GHG) emissions are associated with building operations. GHG emissions are offset by generating clean, renewable energy onsite or offsite.
- 2. Efficiency:** New construction projects consider peak energy while maximizing energy efficiency with a focus on the building envelope and ventilation strategies that drive down thermal energy demand.
- 3. Renewable Energy:** Onsite renewable energy is incorporated into new construction projects to provide added resiliency, minimize offsite environmental impacts, and prepare buildings for a distributed energy future.
- 4. Low-Carbon Materials:** An assessment of the carbon associated with structural and envelope materials—from manufacturing to end of life—informs design decisions.

Key Requirements of the Zero Carbon Building Standard include:

- 1. Demonstration of Annual Zero Carbon Balance**
Central to the program requirements, GHG emissions associated with building operations must be offset with low-carbon renewable energy, either generated onsite or procured through a contractual arrangement.
- 2. Providing a Zero Carbon Transition Plan**
All applicants who rely on onsite combustion of fuels other than zero emissions biofuels must provide a transition plan to demonstrate how the building will decarbonize in the future, showing that they have considered appropriate building design or retrofit measures.

Source: The Canada Green Building Council (CaGBC)

3. Installing a Minimum of Five Per cent Onsite Renewable Energy

ZCB-Design certification requires that at least five per cent of the building's total energy consumption be met using renewable energy that is generated onsite. This requirement does not apply to ZCB-Performance certification.

4. Achieving a Thermal Energy Demand Intensity Target

Thermal energy demand intensity (TEDI) refers to the annual heat loss from a building's envelope and ventilation, after accounting for all passive heat gains and losses. Specific TEDI targets for ZCB-Design certification have been set, which results in greater resiliency and occupant comfort, while ensuring that building designers focus on minimizing a building's demand for energy prior to producing or procuring renewable energy.

5. Reporting of Energy Use Intensity

The ZCB Standard requires applicants to report their Energy Use Intensity (EUI) to provide transparency and enable the industry to learn from each zero carbon building. Reporting EUI also enables the operators of a building to gauge the effectiveness of energy conservation measures and demonstrate progress over time.

6. Reporting of Annual Peak Demand

As with EUI, the rationale for this component of the program is to encourage projects to track and reduce their peak demand over time, to help reduce stress on the electrical grid and avoid the need for additional generation capacity.

7. Reporting of Embodied Carbon

Applicants for Zero Carbon Building certification will be required to report the embodied emissions of their building's structural and envelope materials using life-cycle assessment (LCA) software.

The CaGBC's new Zero Carbon Building Standard represents a bold vision to develop frameworks and tools that will help to dramatically reduce Canada's carbon emissions, and meet its global climate change reduction goals. **AS**

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FEATURE

BRIDGING THE REMOTE

Fort Nelson River Bridge gets a vital makeover

Raj Singh, P.Eng., PE; C.P. (Ken) Rebel, P.Eng.; Chad Amiel, P.Eng.



NORTH



THE LIARD HIGHWAY NO. 77 is a primary route to the Western Northwest Territories and Fort Liard, Nahanni Park, and Fort Simpson. As such, it provides vital access for goods transportation, health and safety, tourism, and emergency access. Along this highway, the Fort Nelson River Bridge traverses the river it's named after, approximately 68km northwest of Fort Nelson and approximately 43km north of the Alaska Highway junction. Due to funding constraints in 1984, the owner constructed only a single lane temporary ACROW superstructure with a timber deck supported on permanent piers that were engineered with capacity to accommodate a future two-lane highway superstructure.

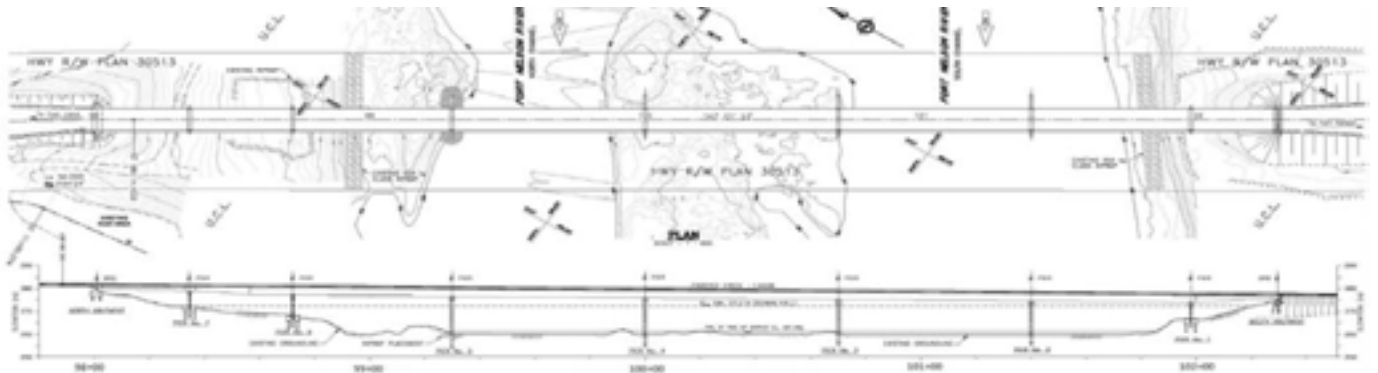
Over time, the single-lane crossing was increasingly causing delays for passenger vehicles and truck traffic travelling to and from the Liard natural gas basin (one of BC's largest reserves), as vehicles needed to wait for traffic to clear in one direction before making their way across the 430m bridge. Therefore, in 2012, the owner,

BC Ministry of Transportation and Infrastructure decided to upgrade the bridge by replacing the existing superstructure to increase traffic volumes and to eliminate delays. The replacement project was procured through a conventional design-bid-build scheme, and McElhanney Consulting Services Ltd. was engaged for the conceptual and detailed design of the bridge on behalf of the owner. The bridge consists of 8 continuous spans of 34.4m – 37.3m – 59.9m – 70.1m – 70.1m – 70.1m – 57.9m – 32.1m, supported on 7 piers and 2 concrete abutments resting on driven steel pile piles.

The team faced several challenges that needed to be addressed in the design of the replacement. Firstly, the substructures and foundations needed to be evaluated based on the current bridge code and strengthened to meet the demands of the new two-lane modern superstructure. Secondly, the climate and remoteness of the site made the durability and longevity of the bridge components important considerations, as maintenance demands have a higher than typical influence



FEATURE



on the lifecycle costs. Solutions requiring less maintenance and more robustness are favourable. Thirdly, engineering the new bridge so that it could be built within the northern region's short construction season was a major challenge to address. Therefore, solutions that allowed construction to continue through the winter or are more flexible vis-à-vis weather conditions were considered more favourable. Finally keeping traffic flowing during construction to minimize disruption to the industrial and public commuters was an important stakeholder requirement.

The team used a multi-account evaluation approach to compare several superstructure replacement options for cost and constructability, as well as suitability for northern conditions, durability, and risk. Thus, steel girders were found to be a more cost-effective solution, considering the bridge span arrangement and site location, transportation, handling, erection, and cost. The 40m sections of steel girders were much easier to handle and transport to site than any other girder solution. Given the relatively long multi-span bridge and limited in-stream access conditions, incremental launching was preferred over

conventional crane erection from a work bridge or ice bridge. In contrast, heavier concrete girders would be difficult to handle and transport in full 70m segments, while shorter segments would require costly post-tensioning operations to splice together. Additionally, erection would require stick-build construction either from the frozen river or a work bridge, ruling out the possibility of girder erection through incremental launching. Steel plate girders with constant depth (or steadily changing depth to accommodate existing pier elevations) were found to provide the best weight and cost efficiency for the site.



“The team used a multi account evaluation approach to compare a number of superstructure replacement options for cost and constructability, as well as suitability for northern conditions, durability, and risk”

The evaluation also considered options for the number of girder lines, to determine an optimal arrangement. While the team considered a four-girder option due to its shallower depth, this would have required approximately 25% more steel and thus was ruled out. The optimal arrangement for structural steel efficiency, and redundancy against collapse, was a three-girder line option. The resulting girder spacing provided comfortable room for inspections and maintenance, a high priority aspect for bridges in northern remote locations. For the bridge deck, partial depth precast and full depth precast options

were assessed; the latter was chosen because fabrication in a certified plant environment would ensure high quality and minimize concrete cover (thus reducing weight,) and the panels could be produced and installed year round. A composite steel girder superstructure with three girder lines and a full depth precast deck was recommended to the Ministry for advancement to the detailed design stage.

To address the project challenges, McElhanney needed to reassess the use of existing substructures constructed in 1984 to accommodate two lanes of traffic. Based on the review of

existing pile capacity information, the team determined that both abutments and the land piers 1, 6, and 7 and the river piers 2, 3, 4, and 5 all required additional piling and pile cap modifications to carry the increased superstructure loads and make them compliant with current code standards. To make the new superstructure compatible with the existing substructure geometry, the three steel plate girders have a constant depth of 3m over most of the bridge with the end spans transitioning down to 1.1m to match up with the top of existing abutment seats and top of the pier 7 pier cap.



To enhance durability and reduce maintenance needs, the bridge superstructure, including the deck, was made continuous over the entire length of the bridge, with expansion joints at the abutments only. Deck continuity shields the girder system from the weather elements and improves the durability performance of the bridge, while enhancing user comfort. Minimizing of joints further reduces the maintenance effort and thereby the lifecycle cost of the bridge. Continuity over a length of 430m was achieved through a novel articulation scheme developed to minimize longitudinal pier deflection under braking loads, minimize restraining forces arising from expansion and contraction during temperature changes, and simplify bearing replacement. The existing piers are extremely flexible in the longitudinal direction, so four middle piers were required instead of only two piers to resist external longitudinal loads. Due to their flexibility in the longitudinal direction, the thermal movements imposed on the fixed piers are easily accommodated with minimal stress in the piles.

Given the relatively long multi-span bridge and limited in-stream access conditions due to the northern conditions, the team designed and

detailed the girders for an incremental launch method of erection. In this construction method, the girders and diaphragms are assembled in a launching bed at one end of the bridge and progressively pushed over the piers to the opposite bank. Several unique details were incorporated into the design to simplify temporary equipment and allow the superstructure to be launched without overstressing the permanent components. Design details included a constant width bottom flange to simplify guide roller designs, a gap in the bottom flange splice plate to allow the rollers to pass through, a relatively stocky bottom flange that can accommodate the high compressive stresses during the launch, and a constant depth girder.

In consultations with the trucking association, the City of Fort Nelson, and local road user groups, it became evident that one lane of traffic during construction was highly desirable. Therefore, the replacement design incorporated traffic staging to minimize traffic disruptions. This scheme involved utilizing the ACROW superstructure as a temporary detour bridge adjacent to the existing highway. For this, the ACROW bridge was laterally slid along the piers onto temporary

A steel girder system was the preferred choice, scoring higher on all aspects of the selection criteria including cost, transportation, handling, constructability, erection through incremental launching, and the ability to be laterally slid with relative ease to allow traffic staging. The significantly lighter weight of steel was also advantageous in minimizing the payload on the existing piers that would have required greater strengthening in the case of a heavier superstructure.

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falsework extensions along a temporary detour road that allowed traffic to pass throughout the construction phase.

The project was awarded for construction in 2015 after a competitive bidding process to Forbes Construction Ltd. that sub-contracted with Rapid Span for structural steel fabrication.

The design process considered the site extreme conditions in developing a cost-effective steel solution that incorporates an advantageous erection scheme for this remote, northern bridge. The design was thoughtfully configured to speed up on-site construction, still delivering a quality bridge with improved durability and reduced maintenance. The continuous superstructure is one of the longest jointless girder bridge in British Columbia. Most importantly, the existing superstructure was utilized not only for construction support during low traffic hours but also allowing the flow of industrial and public vehicles throughout construction. The construction of the Fort Nelson River Bridge superstructure is under progress and anticipated to be completed at the end of August 2017. Once completed, the new two-lane bridge will allow traffic to travel unimpeded along the Liard Highway No. 77 and greatly improve current traffic flow required to support the increasing industrial activity. **AS**

PROJECT TEAM

OWNER: BC MINISTRY OF TRANSPORTATION AND INFRASTRUCTURE

CONSULTANT: MCELHANNEY CONSULTING SERVICES LTD.

CONTRACTOR: FORBES CONSTRUCTION LTD. **FABRICATOR:** RAPID SPAN

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FEATURE

A GATEWAY TO RENE

The John Street Pedestrian Bridge joins two worlds

Aaron Bean, Project Manager, Walters Inc.



WAL



LOCATED IN TORONTO, Ontario, the John Street Pedestrian Bridge creates a new passageway over a busy rail line which in the past divided two Toronto neighbourhoods. As a gateway to a renewed part of the city, it had to be both functional and pleasing to the eye. The John Street bridge is now the new gateway to Toronto's revitalized Old Weston Village, and nicely complements the New Farmer's Market and Cultural Hub projects that surround the area.

Walter Group's role in this project was detailing, fabrication, and the preassembled delivery of the bridge to the project site. Fabrication took place at both Walter's Princeton, Ontario and Hamilton, Ontario plants. The seemingly simple design presented interesting challenges throughout the fabrication and detailing process.

The John Street Pedestrian Bridge consists of two 93-foot-long by 23-foot-high arched trusses constructed of 16 inch diameter round HSS (Hollow Structural Steel) top and bottom chords, and 7/8-inch diameter stainless steel cables for the webs. The bottom lacing of the bridge consists of 8-inch diameter HSS. These 8-inch diameter HSS floor cross beams descend from the truss bottom chord with a curved shape. To ensure optimization of this welded transition, Walters constructed mock pieces which were welded, then sectioned in half, to confirm acceptable weld penetration.

Walters welding engineers created test joints in which two tubes were welded together to create an elliptically shaped weld where the



“It took a collaborative effort by all team members to ensure that the final product matched the original design intent. The finished product is something that we can all be proud of.”

Aaron Bean, Project Manager, Walters Inc.

branch member met the chord member. These joints were then welded and sectioned to ensure adequate weld throat.

There were many challenges in all facets of this project. The final appearance of the bridge trusses needed to be seamless, so each piece forming the arch of the truss needed to be carefully fit and welded in the shop with an AESS (Architecturally Exposed Structural Steel) finish. Compounding challenges even further was the 30-degree outward slope of each arch with no top chord cross support members. This created significant loads in the ends of the trusses which needed to be resolved.

“The John Street Bridge project was particularly challenging due to the design, shape, and logistics of shipping this large bridge to site and pre-assembling it prior to erection,” shared Aaron Bean, Project Manager at Walters Inc. “It took a collaborative effort by all team members to ensure that the final product matched the original design intent. The finished product is something that we can all be proud of.”

There was also an extensive exercise that Walters had to go through when considering the shipping of these trusses. Each side of the John Street Pedestrian Bridge was shipped fully assembled. This required oversized loads that limited when components could



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FEATURE



leave Princeton, Ontario and enter Toronto. Numerous trips to the site to review the route and access points were conducted. During these visits, it was determined that the GO train signal arms needed to be temporarily taken down to allow trucks to make the tight turn into the site area. Detailed shipping plans were then generated and submitted to the City of Toronto for permits. Police escorts were also required to provide a seamless journey.

Moving all of these parts needed to coincide with the field teams' ability to erect the bridge off to the side of the bridge's final position; logistics and communication were key.

The assembly of the two main arches with cables and handrails was done entirely in-house, along with all 70 tons of steel which was galvanized, painted and shipped to the site in two large pieces.

Erection was completed by the end of June 2015, and installation took place entirely

overnight by lifting the bridge by crane over the GO train tracks, which remained live and operational throughout the installation process, while lowering the arches into place.

Walters is pleased to have been a part of this project with our project engineering partners at Parsons, and to have had the opportunity of delivering excellence to our valued client KO Constructors and owner Metrolinx. **AS**

PROJECT TEAM

ENGINEER – PARSONS **GENERAL CONTRACTOR** – KO CONSTRUCTORS **OWNER** – METROLINX **CLIENT** – KO CONSTRUCTORS

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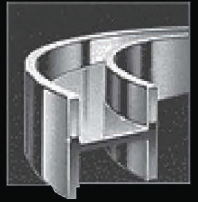


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FEATURE

FLYOVER BRIDGES

NE Anthony Henday Drive and Yellowhead Trail Interchange

Myles Eugene Henry Lewis, P.Eng., Structural Engineer



THE NORTHEAST SEGMENT of Anthony Henday Drive (NEAHD) opened on October 1, 2016 and brought to completion a \$4 billion investment in Edmonton's Ring Road, the first in Alberta. The NEAHD consists of nine interchanges, 47 bridge structures and 27 kilometres of six to eight lanes of divided highway. The Edmonton Ring Road, the largest highway construction project undertaken in Alberta's history, forms an 80-kilometre free-flowing route around the City of Edmonton and connects 24 municipalities in the Capital Region.

The project included a three-level system interchange connecting NE Anthony Henday Drive to Yellowhead Trail. The combination of high traffic volumes, the free-flowing traffic design, a very small land footprint and tight project schedules under the P3 model made the design of this interchange highly complex. This interchange required the modification of five bridges and the construction of 17 new bridges, including two flyover ramps. This required a great deal of collaboration within the project team in bridge design, fabrication, transportation logistics and site erection.

The two bridge structures we are profiling are interchange flyover bridges 23.3 and 23.5. These bridges were constructed with straight plate I-girder segments arranged with a series



FIGURE 1: Anthony Henday Drive / Yellowhead Trail Interchange near completion

Courtesy of FDAL JV

of kinks forming continuous segmentally curved girder lines. Bearings on conventional abutments and intermediate substructures support the steel girders. Figure 1 shows the two flyover bridges within the systems interchange. Structure 23.3 is a

415m long six span (approx. 48-67-92-79-67-64m spans) with a radius of 347m. The bridge forms a S-E ramp connecting southbound Anthony Henday Drive to eastbound Yellowhead Trail with two traffic lanes on a 14.85m wide deck. Structure 23.5 is a 315m long five span (approx. 48-62-86-57-62m spans) with a radius of 340m. A 11.75m wide deck supports one traffic lane that connects northbound Anthony Henday Drive to westbound Yellowhead Trail forming a N-W ramp.

DESIGN

Span Geometry:

Very early in the pursuit phase it became apparent that the pier layout for the curved flyover bridges would be a challenge. Perpendicular crossing

Very early in the pursuit phase it became apparent that the pier layout for the curved flyover bridges would be a challenge.



Courtesy of Rapid-Span Structures Ltd.

FIGURE 2: Bridge 23.5 Girder Fabrication

It was concluded that the curved bridges should be constructed with straight steel plate I-girders arranged with a series of kinks forming continuous segmentally curved girder lines.

arteries, provisions for future lanes on Anthony Henday Drive, 9 – 11m horizontal clear zones, and small medians greatly restricted the physical space available for pier placement. Thus, the site constraints dictated long interior span lengths. The versatility of steel girders permitted an economic design by utilizing continuous girders that varied in girder depth, with deeper girders at the longer span(s) and shallow girders at the shorter spans. Varying the girder depth optimized the girders as well as reduced the earthworks required at the bridge approaches.

Girder Geometry:

Before design work on the project commenced, the consultants, contractors, and steel fabricators met to review the fabrication approach for all bridges. It was concluded that the curved bridges should be constructed with straight steel plate

I-girders arranged with a series of kinks forming continuous segmentally curved girder lines. Fabricating the girders in straight segments made for easier fabrication and transportation of girder segments. Using straight girder segments also had the benefit of reducing the number of primary tension member cross frames to only those cross frames located adjacent to the kink locations. The fabricator joint venture on this project (Rapid Span/Structural) worked collaboratively to determine the most efficient utilization of their fabrication facilities to complete this project. This involved complex logistics that included deliveries by train and truck requiring transloading from three plants.

Flange widths were limited to multiples of 300mm to ensure efficient material use, ripping multiple flanges from a single rolled plate. Flange thickness increments were

generally limited to 5mm to standardize the plate used for all plate girders on the project. The resulting flange sizes varied from 600mm x 30mm to 900mm x 75mm over high demand regions near the piers.

The fabricators’ capacity limited the maximum feasible web depth to 3.7m. Deeper webs would have required a longitudinal web splice that was cost prohibitive. For simplicity and ease of fabrication, girder depth variation was accommodated with linear transitions. The resulting web depth of bridge 23.3 varies from 2.22 to 3.52m, which permitted the camber to be cut from the limiting 3.7m plate. To reduce the number of transverse stiffeners required on bridge 23.3, the web thickness varies utilizing 20, 22, and 25 mm thick webs. The web depth variance on bridge 23.5 is less pronounced at 2.4 to 3.0m. With less incentive to vary the web thickness, bridge 23.5 utilized a constant 18mm thick web.

The total girder tonnage, including cross frames and lateral bracing, is approximately 1,900 & 1,200 metric tonnes for bridge 23.3 and 23.5 respectively.

Speaking to the choice of steel for these bridges, Paul King of Rapid Span and Albert Chiza of Structural stated, “It’s no secret to those in the business that steel bridge girders offer many advantages in terms of versatility, constructability and economy. While only 15 of the 47 bridges on this project were steel, they constituted the longest and the most complex structures. Steel’s versatility easily accommodated the many horizontally curved alignments and highly skewed crossings. This is why steel ended up being used in over half of the bridges, in terms of total length, for this challenging project.”

Field Segment Geometry:

The kink locations were chosen to produce kinked field splices, as shown in figure 2, which eliminates competing efficiencies of placement in low demand regions. Detailing two kinks within each intermediate span and one kink at each end span produced a maximum field segment of 41m for both structures. The heaviest girder segment weighed over 62 metric tonnes. The average field segment length was 37 and 35m for bridges 23.3 and 23.5 respectively.

Cross frames:

One distinct advantage of having the flange noncollinearity concentrated at the kinks is the ability to locally strengthen the flanges for these effects. To increase flange capacity,

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Courtesy of AECOM

FIGURE 3: Bridge 23.3 Girder Cross Frames

while simultaneously reducing the flange lateral bending demand, cross frames are located on both sides of each girder kink. This arrangement greatly reduced the flange unbraced length at the kink locations. Furthermore, the resultant lateral thrust from the misaligned flange forces has a decreased eccentricity to the supporting cross frames.

ARTICULATION:

Both structures are tangentially restrained at the centermost pier support with fixed pot bearings at the interior girders and radially guided pot bearings at the exterior girders. The remaining exterior girder bearings are free in both directions, eliminating radial thermal induced forces. Guided pot bearings at interior girders provide articulation, allowing translation parallel to the flanges.

CONSTRUCTION:

To minimize a lateral bending component from an out-of-plumb girder, cross frames were detailed to fit under total dead load conditions (TDLF). This was facilitated by straight girder segments that are torsionally flexible, allowing the girders to be readily twisted to achieve fit.

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FIGURE 4: Bridge 23.3 Girder Erection (Pier 5 to Abutment 1)



FIGURE 5: Bridge 23.5 Girder Erection (Pier 1 to Pier 2)



FIGURE 6: Bridge 23.5 Girder Erection Completed (Pier 4 – Abutment 1)

FIGURE 7: Bridge 23.5 Completed

Girder segments were erected using crawler cranes and temporarily supported on steel bents during construction. Girder erection started at the centermost pier support with the first girder line segments supported by shimmed bearings and a temporary shoring tower. Temporary restrainers prevented bearing movement prior to grouting the shimmed bearings in place. Girder segments were placed individually, progressing radially. Cross frames were installed incrementally between girder lines as girder erection progressed. Once the four girder line segments were complete with cross frames and lateral bracing, girder erection progressed tangentially down chainage with placing the next four girder line segments. This erection pattern was

repeated until completing girder erection to the down chainage end of the bridge. Afterwards, girder erection progressed similarly up chainage from the centermost pier to the other end of the bridge. Temporary shoring towers supported the girder segments as needed and were removed prior to precast deck panel installation. As seen in figure 6 the shoring towers under the longest span remained in place to completion of girder erection to control girder deflections. Temporary shoring towers under shorter spans were removed once all field splices, cross frames, and lateral bracing were completed within the span. Bridge 23.3 and 23.5 girder erection was completed over approximately 10 weeks and six weeks respectively.

CONCLUSION:

The two fly-over bridges at the Yellowhead Trail and NE Anthony Henday Drive interchange are eye-catching and elegant as they carry all manner of vehicles across their long curved spans. They are an excellent representation of the strengths of designing bridges in steel to meet complex project demands. Design challenges were overcome by working with the entire P3 team to develop innovative solutions to deal with space limitations, height constraints and project deadlines that kept the project on track and allowed the final section of the ring road to be opened to traffic ahead of schedule. The project team, the Province of Alberta and the steel construction industry can be very proud of these elegantly beautiful fly-over bridges. **AS**

PROJECT TEAM FOR BRIDGES 23.3 & 23.5:

OWNER: ALBERTA MINISTRY OF TRANSPORTATION **CONTRACTOR:** FLATIRON-DRAGADOS-AECON-LAFARGE (FDAL) JOINT VENTURE

PROJECT PRIME CONSULTANT: AECOM **DESIGN ENGINEER:** STANTEC CONSULTING LTD. **FABRICATOR:** RAPID-SPAN/STRUCTAL JOINT VENTURE

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AN ESSENTIAL LINK

The design and construction of the Sir Ambrose Shea vertical lift

Hellen Christodoulou, Ph.D.ing., B.C.L., LL.B., M.B.A., CISC Quebec Region Manager



ON SEPTEMBER 23, 2016, the new Sir Ambrose Shea lift bridge in Newfoundland and Labrador, opened to traffic. Located on the Avalon Peninsula in the town of Placentia, approximately 100 km west by southwest of the capital city of St. John's, it was built as a replacement to an existing structure constructed in 1961 and was built directly adjacent to the existing bridge. In addition to being aesthetically pleasing with architecture

reflecting the local culture and tourism potential of the region, the new bridge is designed to be durable, efficient and reliable. The new bridge was constructed adjacent to the existing bridge to minimize disruption to navigation and road traffic. Some of the important design considerations for this new bridge were durability, efficiency and reliability.

The approximate cost of \$47.7 million included construction, engineering and demolition and

removal of the old bridge. The three-span bridge superstructure measures 93 meters in length and includes 1,100 tons of structural steel, including 10 girders for the approaches, and four box girders and stringers for the lift span. The four-hollow structural tubular towers stand over 30.5 meters tall and measure 508mm wide by 25.4mm thick and they are comprised of a three-dimensional steel truss shaped representative of sails, and each tower component is

bridge



connected by a three-dimensional exoskeleton truss housing the machinery operating the lift span. Another welcoming feature was the 1.8m wide pedestrian sidewalk.

The bridge includes 9,200 meters of steel piling, 3,800 cubic meters of concrete and 150 tons of reinforcing steel. The three-span bridge includes a center movable span (vertical lift span) flanked by two simple fixed composite plate girder spans.

“The use of structural steel was instrumental in designing a reliable signature moveable bridge and facilitating its constructability in this harsh natural environment, as well as ensuring its long-term durability.”

Sylvain Montminy, ing., P.Eng., Vice-Président et Directeur Transport et ouvrages d'art - Canada Est, Parsons

THE DESIGN TEAM

FABRICATOR: CANAM-BRIDGES, A DIVISION OF GROUPE CANAM, QUEBEC **ENGINEERS:** PARSONS **ARCHITECTS:** BARRY PADOLSKY ASSOCIATES

DETAILERS: TENCA DETAILERS, QUEBEC **GOODCO Z-TECH:** STRUCTURAL BEARINGS AND EXPANSION JOINTS **CONTRACTORS:** BIRD HEAVY CIVIL & H.J.

O'CONNELL CONSTRUCTION **OWNER:** NEWFOUNDLAND AND LABRADOR DEPARTMENT OF TRANSPORTATION AND WORKS



FIGURE 1: Sir Ambrose Shea Vertical Lift Bridge

As an important part of the regional highway Route 100, this bridge serves as an essential link between the amalgamated communities of Placentia, Jerseyville, Dunville, and Freshwater. It operates year-round, yet the frequency of vessel passage is seasonal and is based on fishing activities, and raised for commercial fishing boats approximately 2,400 times annually, allowing them to enter and leave the Placentia Gut.

Functional requirements for this bridge were important considerations to the architectural features and appearance of the bridge, in view of the bridge's high visibility in the historic community of Placentia, the town's heritage, culture, and local environment and the importance of tourism to the local economy.

DESIGN CONSIDERATIONS

The functional navigational and roadway requirements were key considerations for the new bridge design and alignment. The design of this bridge was in accordance to the requirements of the Canadian Highway Bridge Design Code CAN/CSA S6-06, and related criteria of Section 13, referring to movable bridges, as well as the consideration of loading including: ice, high wind loads, horizontal wind loads and ship collision forces.

The new vertical lift bridge is comprised of three spans; a center 33m lift span, flanked by two 32m approach spans, accommodating two vehicle lanes with a sidewalk on each side. Under the lift span, the clear width of the navigation channel is 25m, with a minimum vertical clearance above high water of 3.05m when lowered and 21.34m when raised. The power and communication utilities were relocated from the existing bridge.

To avoid affecting vehicular traffic and to improve alignment, the horizontal alignment of the new bridge is parallel to the existing with a 22m offset to the East.



FIGURE 2: Both the new and existing lift bridges cross the Placentia Gut

Photo credit: Bird Heavy Civil



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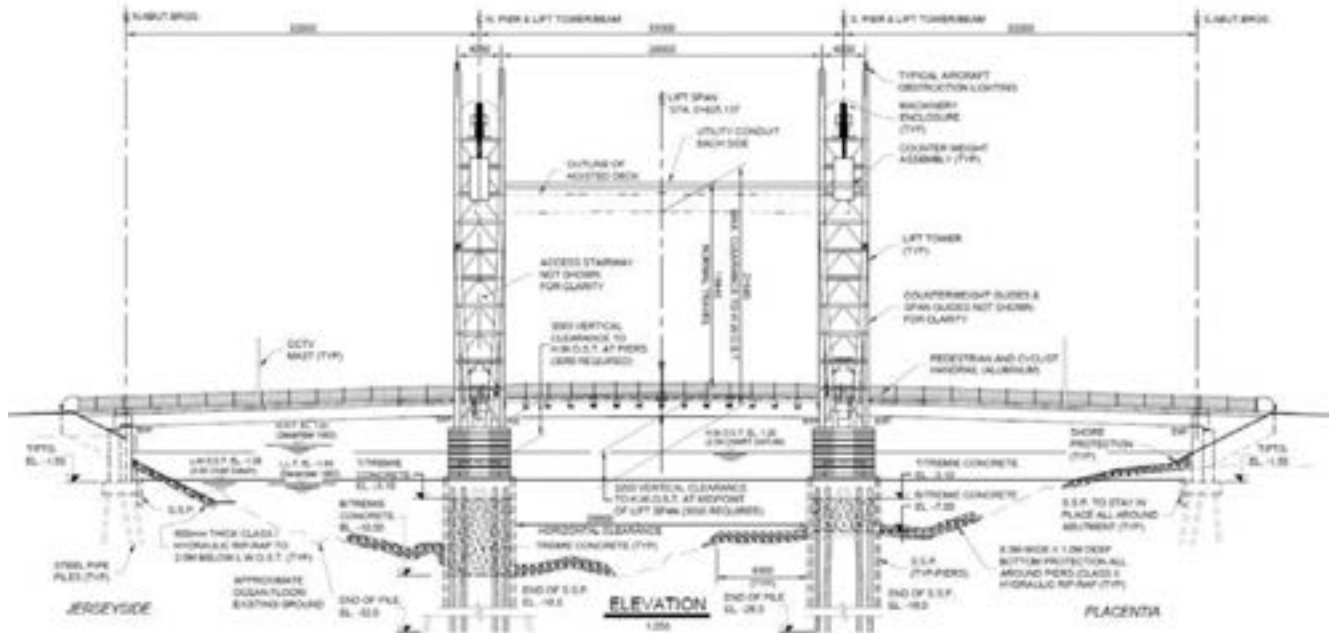


FIGURE 3: Bridge Elevation



Photo credit: Canam-Bridges

FIGURE 4: Bridge is a centre landmark project in the midst of the rich cultural landscape of the Placentia community

ARCHITECTURE REFLECTS CULTURAL LANDSCAPE

From the offset, the Newfoundland and Labrador Department of Transportation and Works had the vision to realize a landmark project in-the-midst of the rich cultural landscape. Apart from its high visibility, this new vertical bridge would be a tourist attraction, as well as an economic development hub. As architectural influence became a key focus of the design concept of this crossing, a means to embody the nautical imagery and complement the surrounding features, with an equally important emphasis on practicality, functionality and durability.

The unique architectural features were inspired by the Salford Quays Vertical Lift Bridge in Manchester,

England and include the two vertical towers on each side of the lift span. Envisioned to represent boat masts and antennas, these aesthetically sleek elements emphasize verticality. Steel tubular members make up this bridge tower silhouette. The transparent machine rooms at the helm of the towers are captivating and attention drawing. To create a focal point of interest and visibility in a harmonious contrast to the dark surrounding hills, sea and sky, the colour choice for the main elements of the bridge superstructure was white. A safe environment and inviting pedestrian experience with a wider sidewalk (1.8m) in comparison with a narrow 1.2m walkway on the existing bridge.

A VERTICAL LIFT BRIDGE WAS SELECTED

Several movable bridge options were considered. A vertical lift bridge was the most viable option, like the existing bridge, incorporating a machinery platform supported at height. Housing the most vulnerable elements higher than grade level and exposure to water, would guarantee better durability for the more vulnerable elements of the bridge.

The priorities in accessing the design considerations, had to seriously consider the reliability of the bridge's operations. For a swing span option, the location of the pier in the middle of the channel would reduce the effective navigable width. A bascule bridge alternative required a counterweight system, necessitating a higher clearance above water, higher approaches and mechanical components housed below the bridge deck.

The reliability of the bridge's operations was a high priority during design. The integration of the mechanical and electrical systems, the standby generators, the Programmable Logic Controller (PLC), used for system control and monitoring for remote monitoring and diagnostics were critical considerations for an effective and efficient operating system.

MANY WERE THE CHALLENGES

Many were the challenges during the construction. The Placentia Gut, a narrow section between the Atlantic Ocean and the river system has over three (3m) meter tidal range with swift currents. Wind conditions, rain and fog were key factors that dictated the direction and project scheduling.

The structural steel was fabricated by Canam-Bridges and shipped from Quebec for erection, so its transportation, though challenging, was exceptionally handled by Canam-Bridges. The

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FEATURE

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FIGURES 5 & 6: Erection using a crane from a temporary work trestle

approximate 100-ton lift span, with some of the mechanical and electrical components of the permanent structure, was erected from a barge. The very complicated erection process required the shutting down of the marine channel.

Design had to consider transportation, erection, and long-term durability. This was achieved by designing the tower members with sealed and welded tubular pipe members. Flange bolted connections were designed to allow the contractor to fabricate the towers in manageable segments to be transported and handled on site without the need for field welding. This option allowed the contractor to fabricate each of the towers in 10 segments, assemble them on site, and erect them using a crane from a temporary work trestle (Figures 5 and 6).

Bolted connections minimized the erection duration of the towers significantly, to a few days in comparison to field welding which would have required substantially more time and a significant window of good weather, which is hard to achieve at this project site.

Durability:

The design team was committed to making the most effective design choices when selecting members, details, and systems to ensure the durability of the structure. The due diligence process included the observation of the existing bridge's performance history, given its constant exposure to very harsh environmental conditions. To ensure durability and optimal, constructability choices included:

- The use of sealed tubular structural sections;
- Opting for enclosures for mechanical machinery and components;
- Positioning of mechanical and electrical compo-

nents in machine rooms 25m above the water level, to reduce the effect of salt spray and salty ocean water exposure;

- Using galvanized reinforcing rebars within the concrete elements;
- Minimizing expansion joints by using semi-integral abutment details at the approach spans;
- The selection of metallization for all structural steel components compounded with a top two-coat paint system as extended corrosion protection.

Utilities:

Existing power and communication lines on the old bridge were transferred to the new bridge using conduits hanging at the side of the approach spans, and up through the tower and into utility pipes specially designed between the towers.

Aesthetics were an integral part of the decision process relating to operational functionality.

The design integrated the mechanical, electrical, and HVAC systems into the structure with minimal impact from a visual perspective while minimizing maintenance requirements and improving overall system efficiency.

BRIDGE SUPERSTRUCTURE

The Approach Spans

For the approach spans, the search was for the best option that would allow the contractor the flexibility to select the best construction procedures, bringing a better value to the owner. After careful consideration, a steel plate girder option with a cast-in-place concrete deck, waterproofing and an asphalt wearing surface was selected. Using a crawler crane from the trestle, the pre-assembled approach span girders, with end and intermediate diaphragms, were erected. Steel provided the best option!

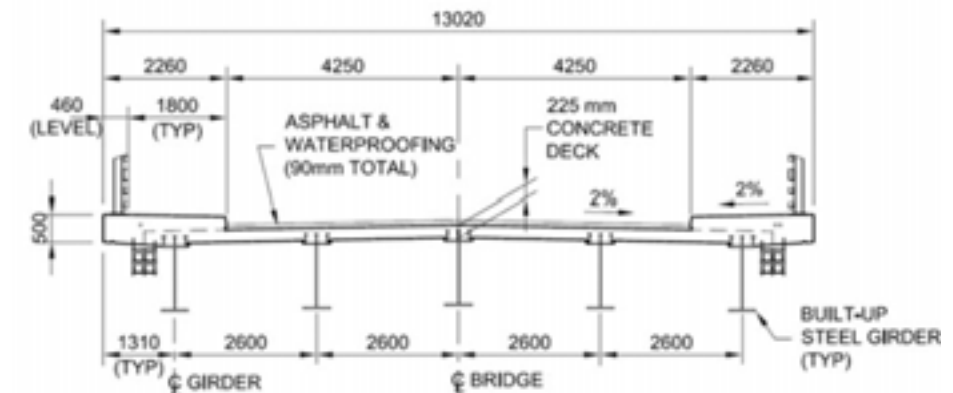


FIGURE 7: Steel plate girders approach span option



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*—Martin Lachapelle, President
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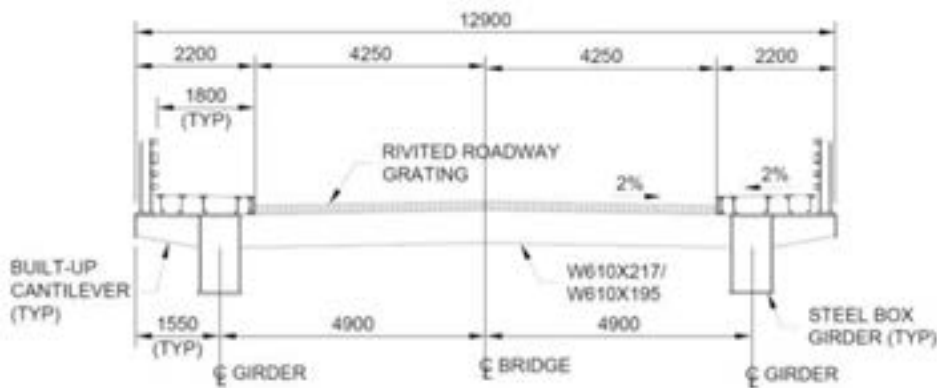


FIGURE 8: Lift span cross-section - open deck grating option

The Lift Span

An orthotropic steel deck was initially recommended for the lift span, however for this instance, the most viable solution of a riveted open deck grating system was selected by the owner.

The framing system shown below is comprised of two main longitudinal box girders with transverse floor beams in between supporting the deck. The lift beams are two box beams that support the longitudinal box girders at the ends of the lift span, which is designed to travel vertically 18.44m. They are used to lift the span during operation with the use of 32 - 38mm diameter wire ropes. The ropes which are connected to a total of four counterweights, each weighing about 50 tonnes, are within the tower and are supported on 3.0m sheaves, as a means to balance the lift span.



FIGURE 9: Lift span framing system

The contractor assembled the lift span, which was designed with optional splices, on a barge from shore, in order to move it into position. The centre span was a major challenge because it involved shutting down the shipping lane for several days to allow the erection of a 100-ton span using strand jacks to lift it into position.



FIGURES 10 & 11: Moving lift span into position

The tight construction tolerances for the movable components and the small deflection requirements for the structural members supporting the mechanical equipment, were an added challenge. All stages of design and construction required extensive multi-disciplinary coordination.

Due to its location in the open surrounded by hills, the bridge was subject to high winds, changing tides, and fast current. These were critical factors considered in the design, to ensure structure stability at all stages of construction, which were also big contributors to limited on-site crane operations. Additional challenges were met since the tide at the bridge site changed direction three times a day and the current reached up to 8 knots.

THREE-DIMENSIONAL TRUSS TOWERS

The towers consist of a three-dimensional truss, shaped to mimic nautical lines as shown in Figure 12. Components of each tower are connected by a three-dimensional exoskeleton truss which houses the enclosure for the machinery. The tower structural members are comprised of closed circular hollow structural sections (HSS) 508mm in diameter for the main tower legs and varying from 168 to 273mm in diameter for the diagonal members. The tower design had to accommodate the counterweights, the counterweight and span guides as well as the access stairs.

The counterweights for the lift span were housed inside the towers and are comprised of built-up steel boxes filled with steel plates. The machine rooms are accessible using stairs located within the towers. An aerial view of one of the towers showing the machine rooms is stunning and an attractive feature of this bridge.

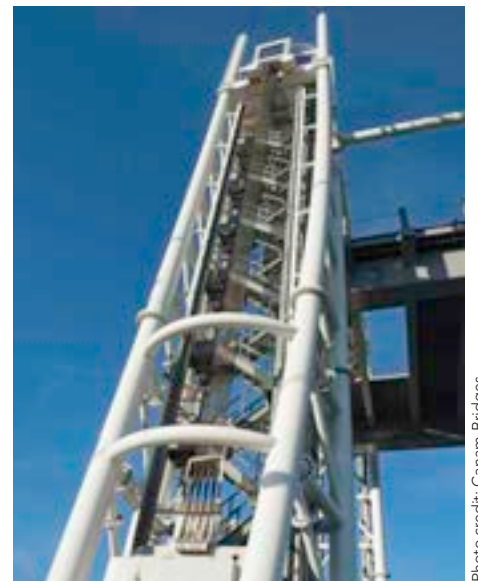


Photo credit: Canam-Bridges

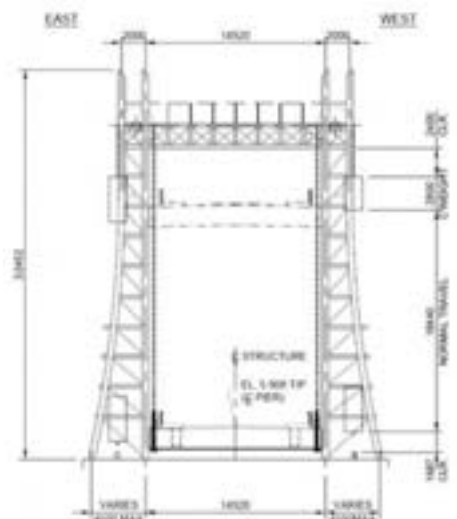


FIGURE 12: Steel tubular towers

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Michel Turcot, Regional Sales Manager, Canam-Bridges

Photo credit: Canam-Bridges



FIGURE 13: Aerial tower view



FIGURE 14: Aerial tower view

Steel Design:

Design of the tower’s tubular connections is not covered in traditional literature or codes, and required extensive finite element models to confirm the capacity of these connections. This was complicated by the fact that numerous loading conditions had to be considered for a movable bridge in both the open and closed position. Figure 15 shows a section of the structural model used to analyze the tower tubular connections. Furthermore, bending the tower legs and braces to the required radii is unconventional and required detailed specifications and testing to ensure the steel tubular member properties were not adversely affected by the bending process.



Photo credit: Canam-Bridges

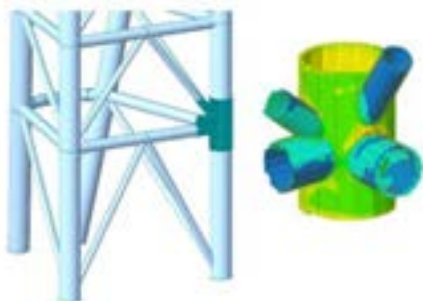
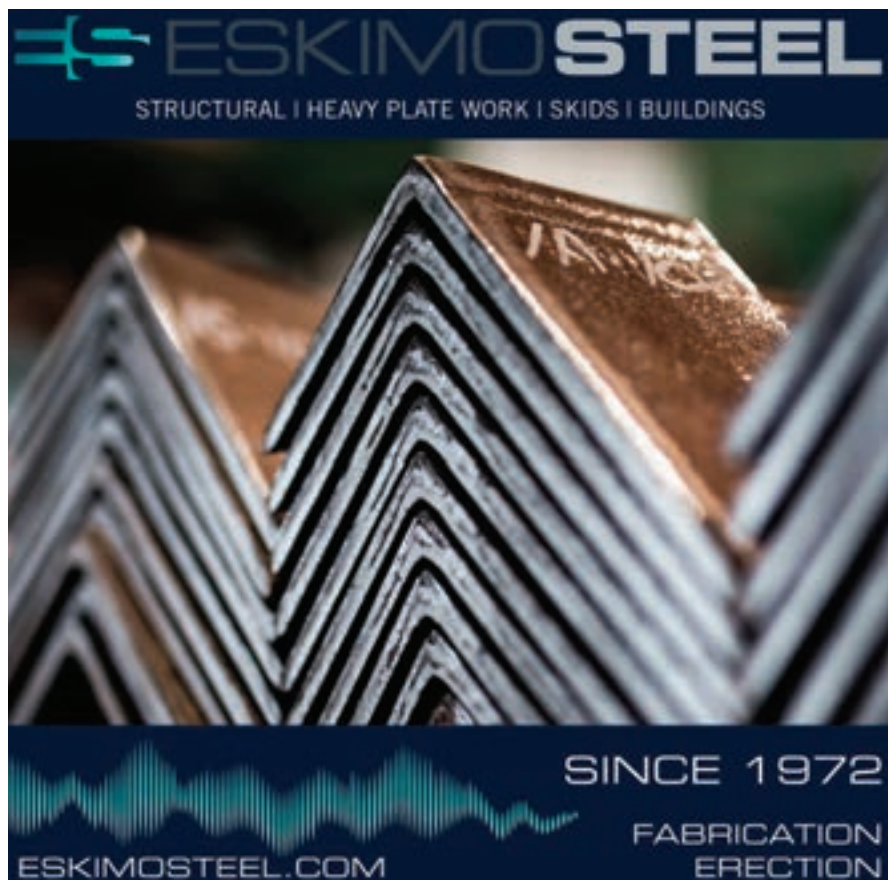


FIGURE 15: Tower finite element model



BRIDGE FOUNDATIONS

Subsurface conditions at the site were of silty and poorly graded sand, looser with depth. The piers are approximately 26.7m in length by 5.7m in width. The size is dictated by the towers and the lifting mechanism.

Design of the piers foundation had to rely on friction piles or a shallow foundation founded on a competent layer with limited bearing capacity. For economical reasons, the contractor elected to build the deep

foundation option. The deep foundation option involved the installation of cofferdams; driving 136 – 324mm diameter close ended pipe piles approximately 30m into the ocean floor; excavating sub-aqueously, the overburden within the cofferdam, down to competent bearing material and pouring tremie concrete; constructing the pile cap on top of tremie concrete and constructing the remainder of the pier in the dry. Pipe piles were selected as they provide a higher

“The new Sir Ambrose Shea Bridge in Placentia, is a remodeled landmark that the townspeople of Placentia can take pride in for many years to come. After being involved in the construction of the Lift Bridge from the tendering phase to the last touch of paint, it has been a challenging project but one that as a team in partnership with the Department of Transportation have worked tirelessly to complete, while enduring some less than favorable conditions. My hat goes off to the execution team and the Placentia townspeople for tolerating all the challenges to deliver the new Sir Ambrose Shea Lift Bridge.”

Darryl Gillingham, P. Eng, Vice President Industrial Development and Energy, Bird Heavy Civil Limited.

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FIGURE 16: Deep foundations using H-Piles



FIGURE 17: Pipe piles at pier foundation

skin friction and end bearing in comparison to H-piles. The deep foundations option is shown in Figures 16 and 17.

The shallow foundation option would have required installation of cofferdams and excavating sub-aqueously to competent bearing material, placing a tremie plug; and constructing the concrete foundations in the dry within the cofferdam. This would have required providing significant bracing for the cofferdams and excavating about 6.5m below the ocean floor for the north pier and 16m for the south pier.

The abutment foundations are supported on 30-324mm diameter friction pipe piles driven

about 16.5m into the soils at the north abutment and 20m at the south abutment.

The abutment and pier piles are closed ended and filled with concrete for added stiffness. At the piers, several pipe piles had to be driven open ended to account for the stiffening of the soil within the cofferdam from the pile driving operations. Access for the piers construction was facilitated by the construction of a temporary work bridge from shore.

BRIDGE OPENS TO TRAFFIC

After 36 months of construction and at an approximate cost of \$47.7 million, including construction, engineering, demolition and

removal of the existing bridge, the new bridge was opened to traffic on September 23, 2016 (Figures 18 and 19).

The aesthetically pleasing bridge has become an iconic structure and an architectural landmark for the town of Placentia, an integral part of the region's touristic hub, as a focal point in the community attracting visitors and new businesses to the area.

The use of steel enabled the designers to deliver a project that surpassed the imposed design requirements, providing the best sustainable option for optimal net positive effect on social, economic, and environmental aspects. **AS**

Photo credit: Canam-Bridges



FIGURE 18: Sir Ambrose Shea Vertical Bridge on opening day September 23, 2016



FIGURE 19: Sir Ambrose Shea Vertical Bridge on opening day September 23, 2016

Continuing Education Courses

WHAT'S NEW - CISC HANDBOOK 11TH EDITION AND CSA S16-14

This 6-hour course offered in nine modules covers changes in CSA S16-14 and the design of steel members and elements using the 11th Edition of the Handbook of Steel Construction. Registrants can take all 9 modules or just the CSA S16-14 changes.

STEEL BRIDGES - DESIGN, FABRICATION, CONSTRUCTION

This course covers the design, fabrication and construction of steel bridges based on CAN/CSA-S6-14, Canadian Highway Bridge Design Code. In addition to 4 reworked design examples, updated topics include code overview, brittle fracture, fatigue, methods of analysis, wind and seismic load effects, and aesthetics including pedestrian bridges.

INDUSTRIAL BUILDING DESIGN

This course focuses on practical and economical solutions for framing a typical industrial building to the requirements of CSA Standard S16-14. The course material will reference the new third edition of the Crane-Supporting Steel Structures: Design Guide and feature a completely reworked design example.

SEISMIC DESIGN OF INDUSTRIAL STEEL STRUCTURES + CSA S16-14 ANNEX M

This webinar presents the seismic design requirements of the National Building Code of Canada 2015 and Clause 27 of CSA S16-14 as these requirements apply to industrial buildings. In addition, CSA 216-14 Annex M is introduced and applied to the design of pipe and process support structures.

SINGLE STOREY BUILDING DESIGN

This webinar focuses on practical and economical solutions for framing a single storey warehouse building with attached office area to the requirements of the 2010 National Building Code of Canada and the pertinent provisions of CSA Standard S16-09.

CISC continues to increase the number of courses and seminars available in the Self-Paced Learning Centre, which offers online education that qualifies for Continuing Education Units (CEUs) using video presentations packaged with notes, design guides, assignments, tutoring and examinations where available. Newly added to the portfolio is the updated Industrial Building Design course and the updated Steel Bridges – Design, Fabrication, Construction course. In addition, look for the discounted bundles of Hot Topic Webinars.

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Current Status and Future Publication Targets

Code/Standard/Supplement/ Commentary/Referenced Document	Current Edition	Next Edition/Revision	Publication Target
National Building Code of Canada (NBC)	NBC 2015	NBC 2020	Dec. 2020
NBC Structural Commentaries (Part 4 of Div. B)	NBC 2010 Str. Comm.	NBC 2015 Str. Comm.	Summer 2017
CSA S16 Design of Steel Structures	CSA S16-14	CSA S16-19	2019
CISC Commentary on CSA S16 (Part 2 of CISC Handbook of Steel Construction)	CISC Handbook 11th Edition ¹	TBA	
CISC Moment Connections for Seismic Applications	2nd Edition ²	TBA	
CSA S6 Canadian Highway Bridge Design Code	CSA S6-14	CSA S6-19	
CSA S6.1 Commentary on Canadian Highway Bridge Design Code	CSA S6.1-14	CSA S6.1-19	
CSA G40.20/G40.21 General Requirements for Rolled or Welded Structural Quality Steel/Structural Quality Steel	G40.20-13 G40.21-13	TBA	
CSA W59 Welded Steel Construction (Metal Arc Welding)	CSA W59-13	CSA W59-18	2018
CSA W47.1 Certification of Companies for Fusion Welding of Steel	CSA W47.1-09 (R2014)	TBA	
CSA S136 North American Specification for the Design of Cold-Formed Steel Structural Members	CSA S136-16	TBA	
CSA S136.1 Commentary on CSA S136	CSA S136.1-16	TBA	

¹CISC Handbook of Steel Construction - 11th Edition includes CSA S16-14, its Commentary, CISC Code of Standard Practice - 8th Edition (new), and design and detailing aids in accordance with CSA S16-14

²Adopted in S16-14 by reference



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CISC MEMBERS & ASSOCIATES HOLD ANNUAL "DAY ON THE HILL" IN OTTAWA ON APRIL 4

A national delegation of CISC Members & Associates were in Ottawa on April 4 to meet with Parliamentarians to urge action on key issues affecting our industry:

1. Support for Fair Payment – Vote Yes for Prompt Payment Act (Bill S-224).
2. Support for Fair Access for Canadian Companies to Canadian Infrastructure Construction Projects.
3. Support to Stop Unfair and Illegal Trade, Trade Circumvention and the hollowing out of the middle class.



CISC MANITOBA / NW ONTARIO HOST THEIR GLITTERING STEEL DESIGN AWARDS & GALA

CISC Manitoba / NW Ontario held their glittering Steel Design Awards & Gala on April 5th at the gorgeous New York Ballroom in the RBC Convention Centre in Winnipeg.

There were a total of 11 awards categories up for grabs at the Award of Merit and Regional Award levels. The award winners included the full team of companies involved in the project. Each Regional award-winning company received a one-of-a-kind CISC engraved trophy as a keepsake. Award of Merit winners received a commemorative certificate and a winning team picture as well.

The evening was attended by a full cross section from the steel construction industry. Regional fabricators, architectural and engineering consultants, steel and other suppliers, educators and students were treated to a multimedia presentation of all the projects and special projects in the region since 2000.

For more details, visit: <http://cisc-icca.ca/awards/manitoba/2017>

CISC WINS ANTI-DUMPING TRADE CASE ON FABRICATED INDUSTRIAL STEEL COMPONENTS



The CISC is pleased with the ruling by the Canadian International Trade Tribunal (CITT) confirming dumping of fabricated industrial steel components into Canada by the People's Republic of China (China), the Republic of Korea (Korea), and the Kingdom of Spain (Spain), and additional subsidizing of fabricated industrial steel components from China.

The CISC will continue to champion Canadian steel and vigorously defend our industry against unfair trade practices for the preservation of Canadian jobs and the success of the Canadian middle class.

Additional information about these investigations is contained in a Statement of Reasons, which is available on the CBSA's website at: <http://www.cbsa-asfc.gc.ca/sima-lmsi/menu-eng.html>.

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BILL S-224, CANADA PROMPT PAYMENT ACT, PASSES THIRD READING IN THE SENATE

The CISC and the Canadian steel construction industry applaud the Canadian Senate for passing Bill S-224, marking a key milestone achievement in our efforts to ensure timely payment for our businesses and over 1.3 million Canadian construction workers and their middle class families.

Bill S-224 will now head to the House for further consideration.

CISC ALBERTA HOSTS DAZZLING STEEL DESIGN AWARDS ON MAY 4TH IN EDMONTON

CISC Alberta hosted the 11th Alberta Steel Design Awards of Excellence in Edmonton on May 4th at the Expo Centre in Edmonton Northlands. 365 guests attended the dazzling Awards Gala.

The finalist in each of the 6 award categories were profiled with individual video vignettes that introduced the audience to the project team, key design and construction details of the project.

Congratulations to all the high calibre Award Winners:

ENGINEERING:

Schulich School of Engineering,
University of Calgary

Fabricator: Supermetal

Architect: Diamond
Schmidt Architects &
Gibbs Gage Architects

Engineer: RJC Engineers

ARCHITECTURE:

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Fabricator: Walters Group Inc

Architect: Kassian Architecture
& Allied Works Architecture

Engineer: RJC Engineers

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Fabricator: Canam Group Inc

Architect: HOK

Engineer: Thornton Tomasetti
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St. Louis Hotel

Architect: Nyhoff Architecture

Engineer: Entuitive

BUILDING COMMUNITIES:

Emerald Hills Leisure Centre

Fabricator: Sturo Metal

Architect: Marshall Tittermore

Engineer: RJC Engineers

STEEL EDGE:

Studio Bell

Fabricator: Walters Group Inc

Architect: Kassian
Architecture & Allied Works
Architecture

Engineer: RJC Engineers

The awards planning committee also held a trophy design competition and selected the winner from four finalists. The trophies were custom fabricated by CISC member CW Carry Ltd. The winning design was based off of AVID Architecture Inc's design.

For more details, visit: <https://cisc-icca.ca/awards/alberta-awards-2017/>

CISC ONTARIO'S STEEL DESIGN AWARDS OF EXCELLENCE WAS A SHOW STOPPING SUCCESS!

CISC Ontario hosted their Steel Design Awards of Excellence at the Art Gallery of Ontario on May 9th. It was a lively evening that started with a networking reception and was capped off by a glittering gala awards ceremony celebrating the best in steel construction

The night started off with The History of Strength Award winners being honoured. Congratulations to: Anne Grimes, Mel Grimes, William "Bill" MacLeod, Terrence "Terry" Nemis, Jimmy Polifroni, Gord Rados and Peter Sheffield.

The show stoppers of the night were the following Award Winners:

PROJECTS CONVERTED OR INNOVATED USING STEEL AWARD OF MERIT:

St. Michael's Cathedral – Toronto, Ontario

CISC Erector: Niagara Rigging & Erecting

Engineer: Quinn Dressel Associates

Architect: VG Architect Ventin Group

Owner: Archdiocese of Toronto

General Contractor: Buttcon

PROJECTS CONVERTED OR INNOVATED USING STEEL AWARD OF EXCELLENCE

**University of Toronto Faculty of Law
Renovation and Expansion – Toronto,
Ontario**

CISC Fabricator: M&G Steel Ltd.

CISC Detailer: M&G Steel Ltd.

CISC Erector: McCormick Steel Inc.

Engineer: Read Jones Christoffersen Ltd.

Architect: B+H Architects

Owner: University of Toronto

General Contractor: Eastern Construction

THE ARCHITECTURAL AWARD OF EXCELLENCE

**University of Toronto School of Architecture
– Toronto, Ontario**

CISC Fabricator: Norak Steel
Construction Ltd.

CISC Erector: Stampa Steel Erectors Ltd.

Engineer: Entuitive

Architect: NADAA / ERA Architects

Owner: University of Toronto

General Contractor: Eastern Construction

**Vaughan Civic Centre Resource Library –
Vaughan, Ontario**

CISC Fabricator: Gensteel

CISC Detailer: Gensteel

CISC Erector: Gensteel

Engineer: WSP Canada

Architect: ZAS Architects

Owner: Vaughan Public Library

General Contractor: Aquicon Construction

CISC Deck Contractor: CANAM Group

THE BRIDGE AWARD OF MERIT

**Hwy 401 Widening and Rehabilitation –
Cambridge, Ontario**

CISC Fabricator: Central Welding
& Iron Works

CISC Detailer: Central Welding & Iron Works

CISC Erector: Central Welding & Iron Works

Architect: Amex Foster Wheeler

Owner: Metrolinx

General Contractor: EllisDon

THE BRIDGE AWARD OF EXCELLENCE

Burgoyne Bridge – St. Catherines, Ontario

CISC Fabricator: Walters Group/ Canam
Group

CISC Detailer: Walters Group

CISC Erector: Walters Group

Engineer: Parsons

Owner: Niagara Region

General Contractor: Pomerleau

CISC Deck Contractor: Vixman
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THE ENGINEERING AWARD OF MERIT

BMO Field Expansion – Toronto, Ontario

CISC Fabricator: CANAM Group

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CISC Erector: CANAM Group

Engineer: Entuitive

Architect: Gensler

Owner: ICON Venue Group

General Contractor: PCL Constructors

CISC Deck Contractor: Vixman
Construction Ltd.

THE ENGINEERING AWARD OF EXCELLENCE

480 University – Toronto, Ontario

CISC Fabricator: Walters Group

CISC Detailer: Walters Group

CISC Erector: Walters Group

Engineer: Sigmund Soudack

Architect: Core Architects

Owner: Amexon Development Corp

General Contractor: Toddglen

CISC Deck Contractor: Vixman
Construction Ltd.

Congratulations to all our Award Winners, and nominees.

For more details, visit: <https://cisc-icca.ca/awards/ontario-awards-2017/>

CISC ALBERTA HOLDS TROPHY DESIGN COMPETITION FOR THE 2017 ALBERTA STEEL DESIGN AWARDS

CISC Alberta held a trophy design competition for this year's Alberta Steel Design Awards. This was an innovative way to further strengthen our connection to the architectural community, and engage them in the Design awards. Four very creative design submissions were received from different architectural firms.

The design criteria the competitors had to meet were that the trophy would have to be light to carry, small enough to handle and must

use steel. The winning design went to AVID Architecture Inc. because their origami inspired trophy design showcased steel beautifully, and was distinct and elegant.

AVID Architecture Inc.'s winning design was then brought to life with fabrication by a CISC member and presented to all the winners of the CISC Alberta Steel Design Awards.

Congratulations to AVID Architecture Inc.



ECOLE DE TECHNOLOGIE SUPERIEURE (ETS) WINS AGAIN WITH OVERALL FIRST RANKING IN THE 2017 CSCE-CISC CANADIAN NATIONAL STEEL BRIDGE COMPETITION & ASCE-AISC'S 2017 NATIONAL STUDENT STEEL BRIDGE COMPETITION

The CISC was proud to be a host and judge at the Canadian National Steel Bridge Competition!

ETS also won 2nd place in the Efficiency category and 3rd place in the Construction Speed, Weight and economy categories in the ASCE-AISC National Student Steel Bridge Competition. Lakehead won 1st place in the Display category.

Congrats also to all the other winning Canadian teams!

ONTARIO GOVERNMENT INTRODUCES BILL 142, THE CONSTRUCTION LIEN AMENDMENT ACT 2017

If passed, Bill 142 will modernize the lien and holdback process, introduce needed rules to ensure Ontario construction workers get paid, on time, for the work they do, and make the dispute resolution process faster and simpler.



NEW CISC WEBSITE LAUNCHED

We're very pleased to have launched CISC's brand new website on June 5.

Our new website has been developed on the latest Wordpress.org platform and is designed to offer new functionalities, and a more visual and user friendly experience.

Our strategy for the website was to put STEEL and our members and associates at the forefront, promoting the strengths of our building material and the capabilities of our members and associates.

Additional new functionalities that are currently under development include:

- a) Advertising opportunities exclusively for members & associates on our News and steelknowledge.ca blogs.
- b) A careers section which will highlight both career streams available in the steel industry and post actual job openings from our members and associates.



CISC HOSTS POPULAR LUNCH & LEARN SEMINAR, "BUILD IT BETTER WITH STEEL" AT ROYAL ARCHITECTURAL INSTITUTE OF CANADA (RAIC) AND ONTARIO ASSOC. OF ARCHITECTS JOINT ANNUAL CONFERENCE, MAY 2017

CISC hosted a Lunch & Learn at the RAIC & OAA Festival of Architecture on May 24th presenting our case for steel: "Build it Better with Steel: Faster, cheaper, greener!" Holly Jordan, a Senior Associate of B+H Architects and Tareq Ali, CISC's Director of Marketing & Communications were the keynote speakers of this event. We had a strong interest from architects with over 35 registered to attend.

CISC showcased steel projects completed by our member and associates to over 1,800 architects in attendance at the conference and the trade show with our presentation and booth.



CISC PROMOTES STEEL AT THE CANADIAN SOCIETY FOR CIVIL ENGINEERING (CSCE) ANNUAL CONFERENCE

We actively engaged with structural and civil engineers, educators and students at our booth at the CSCE conference in Vancouver from May 31st–June 3rd, promoting the possibilities of steel, our new courses, publications and the solutions centre, and the capabilities of our members

and associates. Over 700 civil and structural engineers attended this conference.

We delivered a keynote breakfast presentation on June 2nd on “What’s new at the CISC” promoting our new courses, apps, publications and services to over 300 engineers.



ALBERTA REGION GOLF TOURNAMENT

Join us for the Alberta Region Golf Tournament on Monday, August 14, 2017 at BlackHawk Golf Club!

For player registration information, please contact our event coordinator Shelly Cameron at sch@telus.net

For event sponsorship opportunities, please contact Neil Kaarsemaker, Regional Manager Alberta & Saskatchewan at nkaarsemaker@cisc-icca.ca or 780-934-9557

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Ali Imanpour, University of Alberta, Assistant Professor, Civil and Environmental Engineering

Supplier:

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Alexandra Trovato, NAIT, Professor, Construction Engineering Technology Program

Vulcraft Canada, Inc., 1362 Osprey Drive, Ancaster, ON (Eastern Regions)

Brian Sinclair, University of Calgary, Professor & Former Dean, Faculty of Environmental Design

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Technical Individual Professor (Non-Professional):

Patrick Poulin, Centre de formation des métiers de l'acier, Commission Scolaire de la pointe de l'Ile, Professor, Structural & Architectural Erection Dept.

SASKATCHEWAN GOLF TOURNAMENT

Join us for the Saskatchewan Region Golf Tournament on August 25, 2017 at Moon Lake Golf & Country Club.

For general event information, please contact Neil Kaarsemaker, Regional Manager Alberta & Saskatchewan at nkaarsemaker@cisc-icca.ca or 780-934-9557

SAVE THE DATE: BC STEEL SYMPOSIUM ON OCTOBER 4, 2017

Make sure to mark this date in your calendars! Our panel of presenters will showcase the many advantages of steel construction with valuable resources for architects, engineers, developers, construction managers and general contractors.

19TH CISC QUEBEC STEEL DESIGN AWARDS OF EXCELLENCE GALA

Save the date for the 19th CISC Quebec Steel Design Awards of Excellence on November 6, 2017!

Get ready for a dazzling night of Steel. Don't forget your black tie and formal gown! We look forward to seeing you there! <http://rendezvousacier.com/soiree-gala/>

STEELDAY IS ON SEPTEMBER 15! SIGN UP TODAY!

SteelDay is an annual, national event that showcases the versatility, performance and sustainability of steel and its various innovative applications.

Attend a SteelDay event and learn how the steel industry can support your next project and assist you! Check out www.SteelDay.ca to see what events are near you!

CISC QUEBEC 4TH SYMPOSIUM WILL BE ON OCTOBER 23, 2017 AT ÉCOLE DE TECHNOLOGIE SUPÉRIEURE (ETS)

The theme for this event is "Speed Meeting with the Steel Industry". There will be many mini-technical conferences, plenary sessions and live demonstrations at this conference. www.rendezvousacier.com



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www.apexstructural.ca

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www.jcmdrafting.com

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Les Dessins Trusquin Inc. B, Br
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www.ganawa.ca

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www.sema.ca

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www.payfordsteel.com

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www.arcweld.ca

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http://drsteelinc.com

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www.esfox.com

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http://structuresdebeauce.com

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Acier Picard inc. B
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www.acierpicard.com

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GRAITEC Inc. Longueuil, QC www.graitec.com	450-674-0657	Pacific Bolt Manufacturing Ltd. New Westminster, BC www.pacbolt.com Steel fasteners, structural bolts, anchor bolts, tie rods	604-524-2658	Supreme Galvanizing Ltd Brampton, ON www.supremegalvanizing.com	905-450-7888	Stantec Consulting Ltd., Yellowknife, NT	867-920-2882
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HDIM Protective Coatings Edmonton, AB www.hdimpc.ca	780-482-4346	Peddinghaus Corporation Bradley, IL www.peddinghaus.com	815-937-3800	The Blastman Coatings Ltd. Brampton, ON www.blastmancoatings.com	905-450-0888	CONSULTANT COMPANY	
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Inland Steel Products Inc. Saskatoon, SK www.inlandsteelproducts.com	306-652-5353	PPG Architectural Coatings Inc. Concord, ON www.dulux.ca	905-669-1020	Tuyaux et Matériel de Fondation Ltée / Pipe and Piling Supplies Ltd. St. Hubert, QC www.pipe-piling.com Hot Roll-Wide-Flange-Bearing Pile Beams	450-445-0050	AECOM Canada Ltd., Québec, QC	418-648-9512
Kathbern Management Consultants Inc. Toronto, ON www.steelteamrecruiting.com	416-915-4044	Pure Metal Galvanizing Mississauga, ON www.puremetal.com	905-677-7491	Vectorbloc Corp. Toronto, ON www.vectorbloc.com	416-766-9018	Aecom Consultants Inc., Montréal, QC	514-287-8500
Kubes Steel Inc. Stoney Creek, ON www.kubesteel.com	905-643-1229	Red River Galvanizing Inc. Winnipeg, MB www.redrivergalvanizing.com Supplier of hot dip galvanizing only	204-889-1861	Vicwest Building Products Delta, BC www.vicwest.com Steel metal floor/roof deck, wall and roof cladding	604-946-5316	Amec Foster Wheeler, Vancouver, BC	604-664-5920
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				Vixman Construction Ltd. Rockwood, ON www.vixman.com Roof and floor deck	519-856-2000	Associated Engineering (B.C.) Ltd. Burnaby, BC	604-293-1411
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						BAR Engineering Co. Ltd. Lloydminster, AB	780-875-1683
						Blackwell Bowick Partnership Ltd. Toronto, ON	416-593-5300
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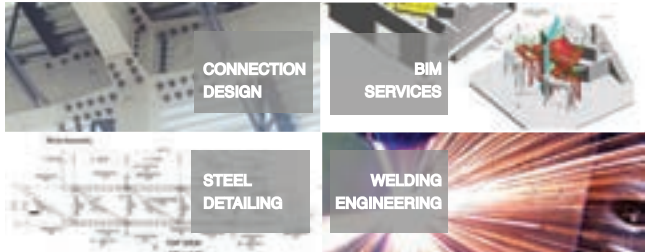

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ACL Steel Ltd. www.aclsteel.ca	67	M & G Steel Ltd. www.mgsteel.ca	6
Advanced Bending Technologies www.bending.net	27	McElhanney Consulting Services Ltd. www.mcelhanney.com	20
AkzoNobel International www.akzonobel.com	33	Moore Brothers Transport Ltd. www.moorebrothers.ca	67
Alberta Steel Awards	36-37	Niagara Rigging & Erecting Company	57
Altitude Steel www.altitude.com	67	Norak Steel Construction Ltd. www.noraksteel.com	11
Apex Structural Design Ltd. www.apexstructural.ca	54	North American Iron Workers Conference www.ironworkers.org	48
Applied Bolting Technology www.appliedbolting.com	43	NUCOR Vulcraft Group www.vulcraft.com	35/47
Atkins + Van Groll Inc. www.atkinsvangroll.com	57	Peddinghaus Corporation www.peddinghaus.com	9
Canam Group Inc. www.groupecanam.com	41	PPG Architectural Coatings Canada www.ppgpmc.com	25
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Haché Technical Services Ltd.	53	Supreme Group LP www.supremegroup.com	Inside Back Cover
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NO 58 SUMMER 2017

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Published by:

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