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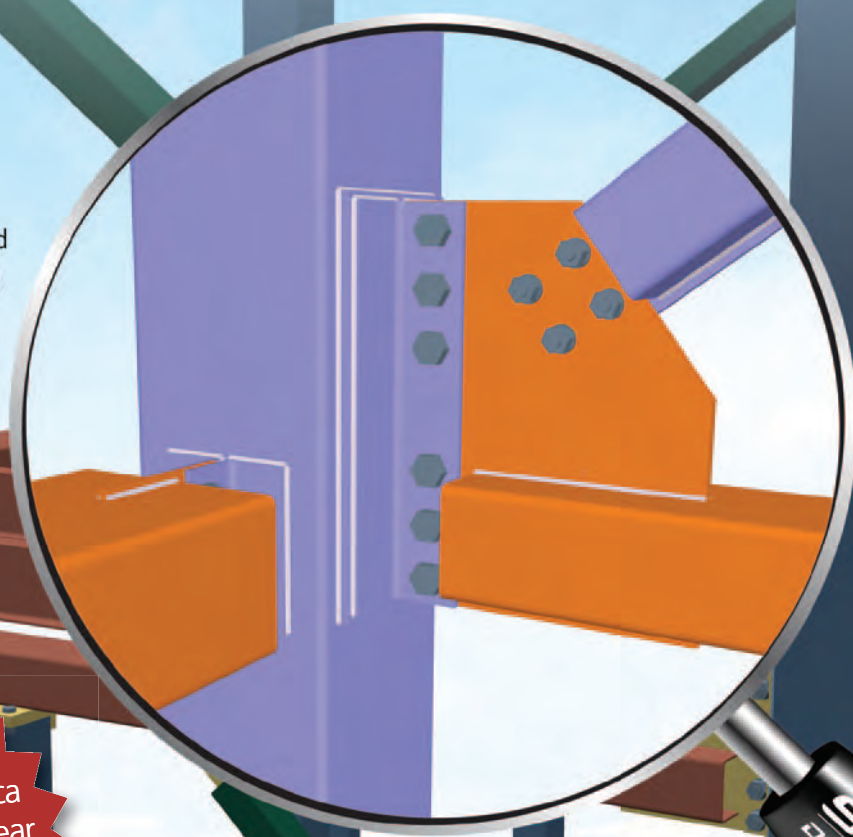
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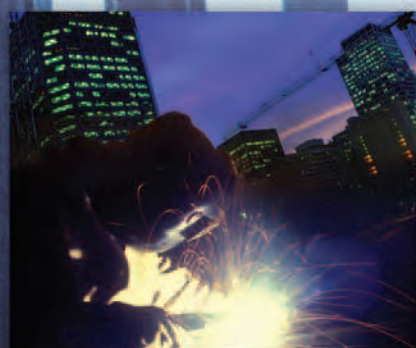
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FROM THE EDITOR

You are probably asking yourself why we have a photograph of New York's iconic new Yankee Stadium on our cover. We are after all, Canadian. Well, Canam Group won a 2009 Quebec Design Award for their work on this, revitalized steel structure. Our article looks at some of the construction challenges faced and the benefit provided

by 3D building information modelling (BIM).

Jeff Packer concludes his series on Elliptical Sections with EHS Connection Design. Many thanks to Professor Packer for this fascinating series. An article on CISC Certification highlights the importance of bringing quality assurance to our industry. Seismic Corner continues its coverage of buckling-restrained braced frames.

We explore the history of success achieved by a series of enthusiastic Dalhousie Engineering students. These students, sponsored by CISC and mentored by Professor Yi Liu have been winning awards for their CANstructions. Yes, they construct projects using cans of food! Afterwards, the food is donated to local food banks.

Not to be missed is our summary of the 2009 BC and Quebec Steel Design Awards. 2009 was one of the best in these regions, illustrating the excellence of steel being fabricated, detailed and constructed by our talented members. Congratulations to all the well deserved recipients including the Robson Square Domes imaged on this page.

This will be the last issue with Ask Dr. Sylvie as Dr. Boulanger is shifting her focus to sustainable steel. Thank you very much for a great run!

Ed Whalen, P.Eng.
President CISC

IN THIS ISSUE

Ask Dr. Sylvie	6
Seismic Corner — Adam Korzekwa	8
Elliptical Sections Three-part Series Part Three: EHS Connection Design — Jeffrey A. Packer	10
CISC Certification — The Structural Steel Industry Controls Quality	12
Home Run Across the Border — Ginette Gélinau	14
CISC Sponsored Student Teams Excel at Nova Scotia CANstruction — Alan Lock	16
British Columbia and Quebec 2009 Steel Design Awards	20
News and Events	24
CISC Members	26

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Professional engineers, architects, structural steel fabricators and others interested in steel construction are invited to enquire about CISC membership. Readers are encouraged to submit their interesting steel construction projects for consideration for inclusion in this publication by contacting CISC.

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COVER PHOTO: The New Yankee Stadium, New York City | Photo courtesy of Structural-Heavy Steel Construction, a division of Canam Group Inc.

PHOTO ON THIS PAGE: Robson Square Domes, Vancouver, B. C. | Ziggy Walsh, George Third and Sons





ASK DR. SYLVIE

Sylvie Boulanger, P.Eng. Ph.D. - Ask Dr. Sylvie is a column for Advantage Steel aimed at readers seeking technical information on steel structures. Questions are welcome on all aspects of design and construction of steel buildings and bridges. Suggested solutions may not necessarily apply to a particular structure or application, and are not intended to replace the expertise of a professional engineer, architect or other licensed professional. Questions for Dr. Sylvie, or comments on previous questions, may be submitted by e-mail to faq@cisc-icca.ca.

THIS IS MY LAST "ASK DR. SYLVIE" COLUMN. I WANT TO TAKE THIS OPPORTUNITY TO THANK ENGINEERS ACROSS CANADA (AND BEYOND) FOR ALL YOUR QUESTIONS, MY COLLEAGUES AND MY NETWORK OF EXPERTS FOR SHARING THEIR KNOWLEDGE WITH ME AND MIKE GILMOR WHOSE IDEA IT WAS TO TRUST ME WITH THIS UNIQUE COLUMN BACK IN 2003!

AVAILABILITY OF 350AT ANGLES

On a bridge project I am working on, I have specified 350AT angles. I have been told that they are difficult to obtain, and that 300 MPa is more common than 350 MPa. Is this true? What are my options? — A.D.

350AT (atmospheric corrosion-resistant weldable notch-tough) steel is more readily available in plates with thicknesses of at least 10mm. They are less available in angles, channels and W shapes. You know that what is most common is 350W steel. So, how do you meet your goal? Find 350A angles and perform a Charpy test (to meet the required 27 Joules energy absorption at -20C in your case) to obtain the T? Or find 350WT angles and use another corrosion system (galvanized or painted steel)? It would appear that it is easier to galvanize 350WT angles than to find 350A angles and test them to get the notch toughness requirement that you are looking for. However, that is not a hard rule. You should contact fabricators in your region to get an idea of what is more common. The question you must have already answered is whether the T is necessary. In many bridge applications, when the members belong to a secondary framing system, the notch toughness is not required.

300 MPa angles are still considered the standard as compared to 350 MPa angles. However, that is changing. Again, availability seems to have a regional flavour. One major nation wide service centre said it keeps about half and half. Another said it kept 80% of its stock in 300W angles. Structural angles are a hot-rolled product. They are more commonly available in ASTM A572 steel but we are seeing more and more ASTM A992 angles especially for seismic design considerations. You should be aware that there is a tendency for service centres to provide 350W angles even when 300W is asked, depending on stock. Make sure the fabricator lets you know of a substitution as this may have an impact on your connection design.

GALVANIZED BOLTS ON WEATHERING STEEL

I am installing a support for conduits on a future bridge. These supports are galvanized angles that will be connected to a 350AT girder. I plan on using galvanized high-strength A325 bolts. Can I use galvanized bolts on weathering steel? — R.L.

Yes, even though a galvanic reaction will occur. According to the AGA (American Galvanizers Association), when galvanized bolts are used on weathering steel, the zinc will initially sacrifice itself until a protective oxidized layer or patina develops on the weathering steel. Once this patina develops, it forms an insulating layer that prevents further sacrificial action from the zinc. The zinc coating has to be thick enough to last until the patina forms, at least two years. Most hot-dip galvanized bolts have enough zinc coating to last until the protective layer develops on the weathering steel, with only a minimal loss in coating life. You should note that A325 bolts are also available in weathering steel grade.

F_y REDUCTION FOR 350W PLATE

In the CISC Handbook (9th Edition, Table 6-3, p. 6-7), there is a reduction in F_y for 350W and 350WT steel (plates, floor plates, bars, sheet, and welded shapes). In fact, for a thickness >65mm, F_y drops down to 320MPa. Why is there an abrupt drop in resistance? And why does that only apply to W and WT steel, and not to A or AT steel? — B.V.

The decrease in tensile properties with increasing thickness is caused by the fact that the hot-rolling temperature is higher and the rate of cooling after rolling is slower on thick sections than on thinner sections. The drop in tensile resistance for W steel appears to be a compromise between achievable chemical composition and weldability at reasonable cost. And the lack of drop in A steel appears to have something to do with its superior cooling process. According to one technical source, the thickness breaks



photo: Terri Meyer Boake

in G40.21 are based on breaks that existed in ASTM specifications for structural steels, which were developed much earlier based on many years of tensile testing. The tensile properties on hot-rolled steel products are dependent on the chemical composition. Within the permissible ranges of chemistries, which are limited by the need to maintain good weldability without the need for preheat, the tensile properties decline with thickness of the finished products. The chemistry of the rolled product can be adjusted within these limits for thicknesses up to approximately 65 mm thickness. Above 65 mm in thickness, the chemistry limit results in lower maximum tensile properties, which is why a lower value of 320 MPa has been included in the specification. To achieve the same tensile resistance, additional alloying or a more complex cooling process would have increased the cost of the 350W steel. This was less of a problem for the higher cost atmospheric steel, which is considered to have a superior chemical composition and cooling process already built into its price structure.


ANCHOR ROD REDUCED CAPACITY

Is there some kind of rule of thumb that can help me evaluate the reduced capacity in tension of an anchor rod as a function of the unused threads in the nut?

According to Gilbert Grondin, professor at the University of Alberta, there is a definite reduction in tensile capacity when the bolt (or rod) does not fully utilize the depth of the nut. This of course is due to the stripping of the threads. During a research project at the University of Alberta, it was determined that a reduced resistance occurs even if only one thread is unused. Unfortunately, there does not seem to exist information on the actual reduced capacity as a function of the number of unused threads. Also, the results are not representative of anchor rods, which have a diameter larger than what was used during this study. As far as bolts are concerned, one can use the full resistance of the bolt provided the last thread used is at least flush with the nut.

If you have found this column useful over the years, I would be happy to hear from you. My email is sboulanger@cisc-icca.ca. As a bonus, I will send you a pdf document of all the questions I've written for this column since 2003. All your questions should now be directed to faq@cisc-icca.ca.

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SEISMIC CORNER

BUCKLING-RESTRAINED BRACED FRAMES – PART 2

Adam Korzekwa

The first part of this series on buckling-restrained braced frames (BRBF) presented the system and its main advantages. The second part of the series presents the specific design approach for BRBFs and their unique qualification testing requirements.

Buckling-restrained braced frames incorporate buckling-restrained braces (BRBs) as primary seismic-force-resisting elements. The design of BRBs is unique since the provisions of Clause 27 in CSA S16-09 only provide guidance for designing part of the system: the steel core. In order to verify the adequacy of the system preventing buckling, physical testing or adoption of reported test results of similar braces is required.

DESIGN OF BRBS

The steel core, cut from a plate, typically has a dog-bone shape with a yielding segment. For a given frame geometry, both the length of the yielding segment and its cross-sectional area (A_{sc}) can be set independently. It is thus possible for the designer to tune independently the resistance and stiffness of the BRB by adjusting the relative length and area of the yielding segment, as illustrated in Figure 1. Braces can be sized for the required resistance at each storey, thus minimizing overstrength and helping the yielding distribution over multiple storeys.

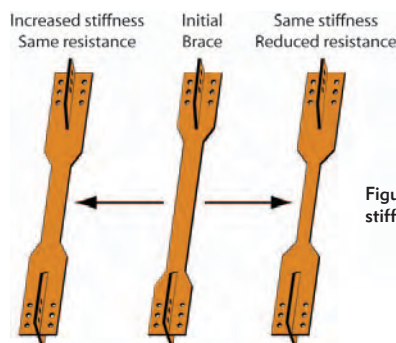


Figure 1 – Adjusting BRB core stiffness and strength

The sizing of the core according to S16-09 is simple and based on a single design equation: $T_r = C_r = \phi A_{sc} F_{ysc}$

This equation assumes an equal resistance in tension and compression and permits the use of the yield strength of the steel core (F_{ysc}), as determined from a coupon test. At the design stage, the core plate dimensions and steel grade are specified. Since the yield stress for a steel grade can vary considerably, possible brace overstrength must be considered in the design. A permissible range of yield strengths can be specified by the designer to reduce the possible variations in brace resistance. Alternatively, the core area can be adjusted to meet capacity requirements according to the results of a coupon test, in which case brace stiffness could vary.

Unlike sizing the core, the BRB resistance for capacity design cannot be assumed equal in tension and compression. The probable

resistance of BRBs is higher in compression than in tension due to Poisson's effect and friction between the core and the buckling-restraining system. This overstrength in compression must be considered in the frame capacity design, since it will affect connections and capacity protected elements, as shown for a beam in a chevron configuration in Figure 2(a). In accordance with S16-09, the probable resistances must be based on results from brace testing.

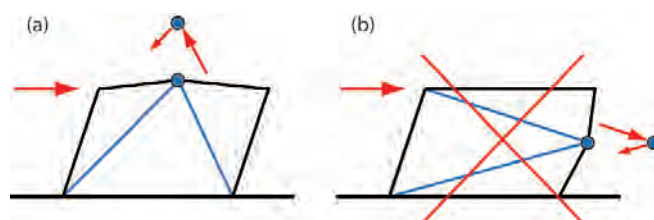


Figure 2 – Unbalanced forces due to brace overstrength in compression

Because of the higher resistance in compression than in tension, K-type bracings are not permitted (Fig. 2(b)). Also, to avoid soft-story response, S16-09 limits the height of BRBF systems to 40 m unless $I_e F_a S_o(0.2)$ is less than 0.35 or a stable inelastic response is demonstrated.

TESTING REQUIREMENTS

CSA S16-09 does not provide requirements for the design of the buckling-restraining system encasing the core nor for other brace details. The standard provides performance-based requirements and the suppliers are usually involved with the detailed design. Results from physical tests are required to demonstrate the system's inelastic cyclic performance and to obtain the resistances used in capacity design. CSA S16-09 refers to the qualification testing requirements outlined in AISC 2005 Seismic Provisions. In those provisions, a minimum of two cyclic tests are required, including at least one sub-assembly test with connection rotational demands. To be qualified, braces must satisfy strength and inelastic deformation requirements. They must exhibit, within the required protocol range, a stable hysteretic behaviour with positive incremental stiffness.

Physical testing needs to be representative of the brace used in the project but need not be done on a project-specific basis. While a coupon test of the core plate is always required, brace qualification testing might not be necessary. The design can be based on tests reported in research or documented tests performed for other projects. Similar strength, design methodology, fabrication tolerances and connection detailing are required.

Tests covering various configurations are reported in the literature, and the body of available test data is expected to grow, which will reduce the need for additional testing. Several foreign BRB manufacturers have pretested proprietary products that could be used without further testing. Some Canadian fabricators have also produced, tested and installed BRBs.

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ELLIPTICAL SECTIONS THREE-PART SERIES PART THREE: EHS CONNECTION DESIGN

Jeffrey A. Packer

In Part II, we examined member design of elliptical sections. In Part III, the last of the series, we take a closer look at connection design of EHS. As with member design, every attempt is made to simplify and rely on what is familiar. In fact, many conventional limit state failure models can still be applied to EHS connections. For example, research on gusset plate to slotted end EHS welded connections (Figure 1) has shown that the failure modes of circumferential fracture of the tube and tear out (or “block shear”) of the tube – well recognized for slotted end CHS and RHS connections – are still applicable to EHS too, and efficient design recommendations have been advocated.

Directly welded tube-to-tube connections always represent a potential problem in tubular construction, due to the high flexibility of the hollow section walls, and recently some tests on welded EHS-to-EHS connections have been performed. Bortolotti et al. and Pietrapertosa and Jaspart undertook the first laboratory tests, in Belgium, on truss-type N- and X- connections, with EHS branches welded to the wide side of the EHS chord, followed by numerical modelling of the same connections. Choo et al. in Singapore extended the finite element modelling of EHS-to-EHS X-connections by studying branches welded to both the wide and narrow sides of the chord, and with the branch also oriented in both orthogonal directions for each chord orientation.



A recent study in Canada on EHS connections consisted of branch plates and through plates (both longitudinal and transverse) welded to both the wide and narrow sides of an EHS chord member (Willibald et al.), as shown in Figure 2. In the analysis of these tests, the notion of using “equivalent CHS” or “equivalent RHS” dimensions for transforming elliptical shapes was attempted. The design of CHS and RHS welded connections is now based on over 40 years of international research, so the prospect of repeating this research volume for EHS members is daunting – hence the quest to relate the design of welded EHS-to-EHS

connections to other well-established design procedures for hollow steel sections.

FUTURE RESEARCH

Interest in EHS behaviour in compression has also now extended to concrete-filled sections (Yang et al.; Zhao and Packer) and even stainless steel oval sections (Theofanous et al.). As might be expected, it has been shown that concrete-filled EHS provide much greater axial load capacity, and greater ductility, compared to empty EHS; a feature that can be employed with EHS columns perhaps, if the upper size range proves insufficient. Research on welded EHS-to-EHS connections is still continuing at the University of Toronto; this is currently focussed on truss-type T- and X-connections (Figure 3) and is being performed by Ms. Tarana Haque. So, architects may soon be venturing into exotic elliptical tube trusses and even bolder AESS statements.

ACKNOWLEDGEMENTS

Financial support for the study of Elliptical Hollow Sections has been provided by the **Steel Structures Education Foundation (SSEF)**, the **Comité International pour le Développement et l'Étude de la Construction Tubulaire (CIDECT)**, and the **Natural Sciences and Engineering Research Council of Canada (NSERC)**.

The full list of References and the complete Table of Engineering Properties for the 46 EHS shapes are available on the CISC website: www.cisc-icca.ca/publications/advantagesteel/36

Jeffrey A. Packer is Bahen/Tanenbaum Professor of Civil Engineering at the University of Toronto.



Figure 1: Gusset plate-to-slotted EHS connection under axial tension – slot end open (University of Toronto)



Figure 2: Longitudinal and transverse plate-to-EHS connections (University of Toronto)

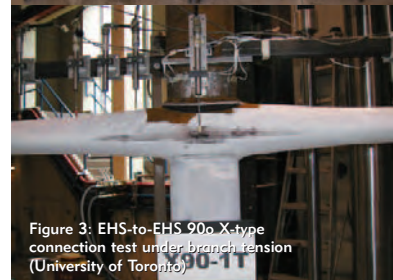


Figure 3: EHS-to-EHS 90° X-type connection test under branch tension (University of Toronto)

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CISC CERTIFICATION – THE STRUCTURAL STEEL INDUSTRY CONTROLS QUALITY

The CISC announced in March its plan to offer a Canadian structural steel quality certification program. Ed Whalen, President of the CISC stated that this program meets a specific need within Canada. "Not all projects demand this level of proof of control, however the move towards quality systems within the construction industry is now becoming common place".

"Through the leadership of the CISC, the steel industry is responding to a need. The steel industry is now moving towards quality programs where other industries have done so years ago", Mr. Whalen said. "We are seeing it all around the world. In Europe with EN 1090 and in the US with the AISC Certification, structural steel specific quality programs are now being developed and mandated."

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- 1) Steel Structures; and
- 2) Steel Bridges

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- 2) Certified companies will be posted on the CISC Website.
- 3) The quality programs will adopt the core principles of ISO 9001.
- 4) It requires structural steel specific procedures and practices to comply with the CSA S16, W47.1, W59, and/or CSA S6.
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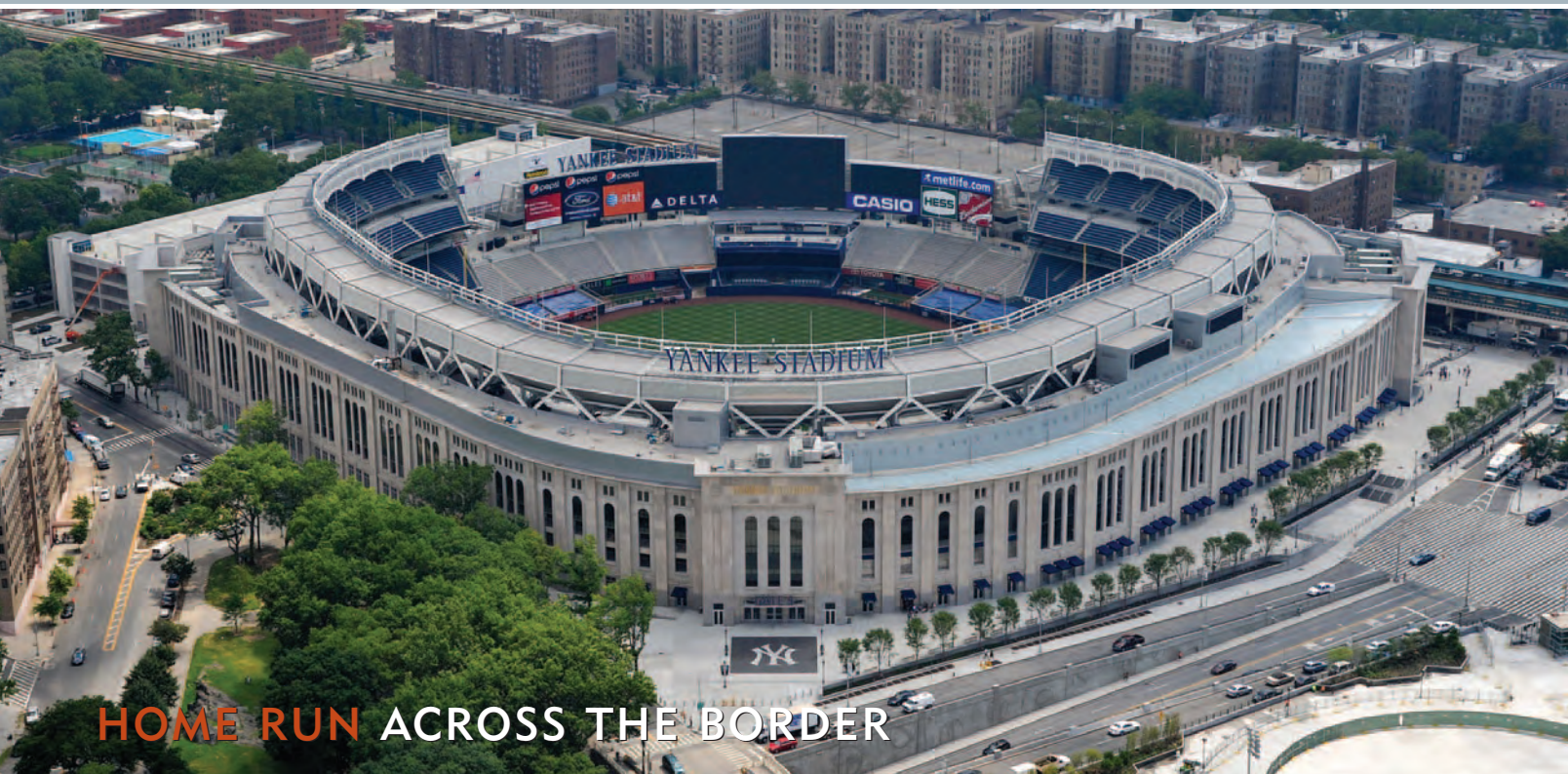
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HOME RUN ACROSS THE BORDER

Ginette Gélneau

Since 1993, Canam Group, headquartered in Quebec, and its business unit Structural-Heavy Steel Construction (Structural) have participated in the construction of 54 stadiums and arenas in North America, 20 of them home to Major League teams. But 2006 was the year Structural hit a home run that started a series of consecutive victories, with the construction of many stadiums and arenas in New York, New Jersey, Pennsylvania, Oregon, Florida and British Columbia. This home run came about with the awarding of the contract to build the steel structure for Yankee Stadium, a 51,800-seat baseball stadium that is very similar in design to the original facility built in 1923. The new stadium, which opened in time for the 2009 baseball season, saw the Yankees win their 27th World Series championship against the Philadelphia Phillies.

STRIKING FEATURES

The new building has two striking features – the legendary frieze adorning the stadium's perimeter and the Great Hall with a majestic simplicity that inspires awe.

THE FRIEZE

Both a symbolic and historic element, the copper frieze which adorned the old stadium was reproduced, but the new one is made of steel and coated with zinc to protect it from rusting. More than just a decorative element, the frieze is part of a system of cantilevers holding up the upper deck and the lighting system on the roof.

The frieze is 446.5 metres long and composed of units each weighing 13 tons between the supporting frames. It is formed from a steel plate 16 mm thick, in which the arc shapes and openings were cut out with a laser. The raised patterns were created from 200 x 150 mm hollow structural sections (HSS).



Installation of a section of the frieze, a symbolic element to the Yankees fan base

THE GREAT HALL

The Great Hall covers 93,000 m² and features a seven-storey-high ceiling. Fans passing through this main entry space can admire the twenty or so banners of past and present Yankees superstars. The banners are framed with architecturally exposed steel, composed of tubular pieces welded from plates and connected to the ceiling and the upper part of the walls.

The Grand Hall's perimeter wall is supported by box columns acting as a perimeter moment frame, with the perimeter frame tied back to the main structure with built-up exposed box girders.

STRUCTURAL DESIGN

Yankee Stadium has five seating levels: Field, Main, Suite, Upper and Grandstand. Intended to maximize the seating space and offer unobstructed views at the Suite and Upper Levels, the structural design called for cantilevers extending up to 15.2 metres, supported behind the last Main Level columns. Steel was naturally the material of choice for this prestigious project. Concrete construction was only used for the Field and Main Levels.

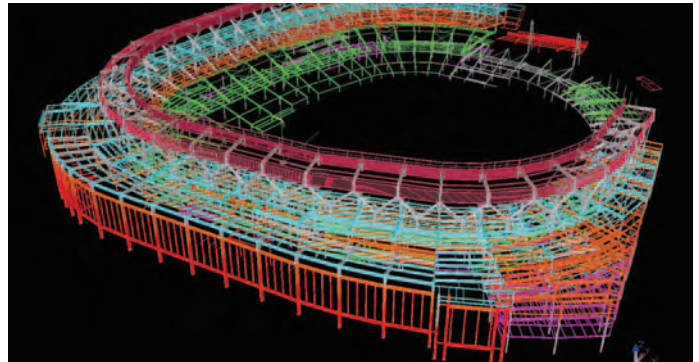
"A project of this magnitude necessarily involves its share of challenges, with or without a hybrid structural concept," explains Serge Dussault, vice-president of engineering at Structal. "Because the lower levels are built of concrete, the major structural steel pieces had to be connected to the concrete walls by means of large embedded steel plates. Since the construction tolerances of concrete are much looser than those of steel structures, we had to use connections that could adjust quickly to highly variable site conditions," he adds.

USE OF 3D BUILDING INFORMATION MODELING

According to Michael J. Squarzini, P.E., LEED AP, senior engineer with Thornton Tomasetti, since work progressed at an accelerated pace, the use of 3D building information modeling was critical to detecting clashes between the various trades and allowed the team to achieve project-wide coordination and keep the project on schedule.

Ensuring that half of the structural components remained exposed and part of the project's architectural structure posed a challenge, because the changing geometry along the stadium's radial axes required many single-use connections that had to meet high aesthetic standards.

"The 3D model became an indispensable tool for reviewing the connections, particularly the frieze connections, since the frieze is both an ornamental and structural element of the new stadium," Squarzini adds.



3-D model used at different stages of construction

RECOGNITION AND STEEL DESIGN AWARD

Yankee Stadium received an Honorable Mention at the 2009 CISC Steel Design Awards for successfully tackling many technical challenges associated with the steel-framed structure.

At the Quebec Steel Symposium, which preceded the CISC Steel Design Awards, Yankee Stadium was one of the finalists in the annual competition and, as the members of the Structal team proudly learned, won the People's Choice Award.

Yankee Stadium also made a great impression on our American neighbours as evidenced when the project was recognized as Overall Project of the Year by *New York Construction magazine*.

"As a structural steel contractor," states Luc Pelland, president of Structal, "we play a vital role in all the steps needed to achieve successful project integration. Our customers know they can count on our proven expertise and our commitment to serve them well."

OWNERS: New York Yankees
ARCHITECTURAL FIRM: Populous (formerly HOK Sport)
ENGINEERING FIRM: Thornton Tomasetti
GENERAL CONTRACTOR: Turner Construction
STRUCTURAL STEEL CONTRACTORS: Koch Skanska/Structal-Heavy Steel Construction

3D MODELS AND DRAWINGS: Technyx, Cadmax
STEEL FABRICATOR: Canam Group
TONNAGE: 13,000 tons
FABRICATION PLANTS: St. Gédéon de Beauce, QC
Quebec City, QC

CISC SPONSORED STUDENT TEAMS EXCEL AT NOVA SCOTIA CANSTRUCTION®

Alan Lock

CANstruction Nova Scotia a Feed Nova Scotia community event, helps fight hunger in a way that is both educational and fun. It is an opportunity to showcase remarkable talent from the region. For the last five years CISC has been a proud sponsor of teams from Dalhousie University in the annual Nova Scotia CANstruction event. During this period the CISC sponsored Dalhousie Civil Engineering teams have constructed winning structures for a total of seven awards.

Teams of architects, engineers, designers and students mentored by professionals, compete to design and build structures made entirely from full cans of food. The cans used may only be held together using scotch tape, cardboard and rubber bands if necessary to form the structures. Imaginations run wild as participants are challenged to use their knowledge and skills to stack cans into structurally sound objects, often exploring uncharted territories in engineering and architectural design. Thanks to the creative genius of the teams, fantastic "CanSculptures" gradually emerge from what at first looks like organized chaos. The entries are judged by a panel of community leaders and local professionals. Winners are chosen in each of the six different categories. The Award Categories being:

- Best Meal
- Structural Ingenuity
- Best Use of Labels
- Jurors Favourite
- Honourable Mention
- People's Choice Award for each location
(as chosen by the public)

The resulting "CanSculptures" are displayed for several days in various public places. At the close of the competition, all of the canned food used in the structures is donated to Feed Nova Scotia for distribution to its many member agencies - including food banks, shelters, soup kitchens and school lunch programs. In 2009, Feed Nova Scotia received 18,476 kg of food (33,653 cans) from CANstruction Nova Scotia participants. 17 teams participated, raising approximately \$41,000 to help feed hungry people throughout the province.

Photos of the winning entries are sent to an international judging competition in the United States, where they compete with the best structures from over 147 cities throughout the world. In 2005, the Dalhousie civil engineering teams entry "The Two-faced Grecian Vase" mentored by Dr. Liu, received an honourable mention in the International CANstruction competition held in Los Angeles.

All of the CISC sponsored past entries in the competition have highlighted diverse, complex and intriguing designs; a few of these entries are shown below.

2009 – DEFYING GRAVITY, DEFYING HUNGER

Award – Structural Ingenuity



The team left to right:
Stacy Murray, Tracy Walker, John Bergese,
Sandra Soon, Professor Yi Liu,
Mingqing Deng, Whitney Surgenor

The introduction from the team explains their purpose. "The 'gravity-defying' can-sculpture is our way of helping to eradicate hunger's painful and persistent effects in our society. With your very kind monetary contributions you 'can' all join in with us as partners in 'can-sculpturing' to help defying hunger". The fun and the challenge of building these complex shapes has been the "drawing card" for the students, some being involved on previous year's teams, while at least one past team member took part in three competitions.

Dr. Liu spoke of her involvement, "I just wanted to have a great structure and I think that we have achieved that and most importantly the students had a great time".

2007 – THE CURL

Awards – Structural Ingenuity
– People's choice

The Curl

*You may not have heard of all the curls of the world,
That arise in demand over our great global land;
A little child's curls with their ruffles and whorls,
Put joy in our hearts much more than a grand;*

*We will not forget the young children with curls,
And the hurt hunger causes in their little world*

Yi Liu, Associate Professor, Civil Engineering



Team members included students Josh DeYoung, Alan Grant, Sandra Soon, Lynsey Poushay, Sabine Chantel, Susan Tibbo, Julie Briand, and Dane George

In 2007, the Dalhousie engineering students defied gravity and expectations with an uncanny contribution to CANstruction Nova Scotia. Mentored by Associate Professor Yi Liu, the civil engineering team won the structural ingenuity award and the Halifax Shopping Centre people's choice award for their contribution, "The Curl," was assembled using 2,000 cans of tuna. Nineteen teams participated in the exhibitions, which were held at different shopping centres in the province. Shoppers knelt on the floor to try and figure out how the cans were held in place for "The Curl." Although somewhat unassuming, the structure was appreciated for its flowing lines and the complexity of its design.

It was an intense project for the busy students and professor. "It gives the students a sense of awareness of the needs within the community, and their responsibilities toward helping to solve problems in society, in this case, hunger," says Dr. Liu. "Feed Nova Scotia is a good cause, and we all have fun as a team. This competition also pushes us to think of structural designs we would never be able to work on in the normal course of our education and careers.

2005 – I AM AMPHORA,
THE TWO-FACED GRECIAN VASE

Awards – Honourable Mention Award

– Honourable Mention in the 'International
CANstruction Competition, held in Los Angeles.

(The only Canadian winner at this level to date)

*I am a Grecian vase from way back in antiquity,
My ears, my foot, my neck, and my mouth,
Are all the parts my makers named for me.
But I am a little proud, I most honestly do say,
I show my faces on all sides and they are not made of clay.
My only foot is on the ground and spread a little all around,
That helps me stand and then my body grows,
As your gaze moves upwards from my toes.
My body is a shell you know, that's made of stacked up cans,
A feat that isn't easy if you've ever balanced on your hands.
Then my lipped mouth is supported by my pretty neck,
A cantilevered lip around my mouth that really caused us heck.
I was a trial of love and labour by Dal students for all you to see,
You can find all my faces, but not the one inside of me!*



Team members included students Korapin Chaotakoongite, Neal Cody, Tyler Chapman, Marcia Smith, Matt Himmelman, along with Professor Yi Liu, Jeff Clair and Mark MacDonald.

This ingenious event showcases the remarkable talent and creativity within the community, as well as aiding in the fight against hunger. All team members involved have benefited from the sharing of ideas in the original designs and constructability of these structures under the constraints of the competition. The CISC Atlantic Region continues to be proud to share in the development of young engineering minds of the future.



BUILDING THIS BRIDGE COSTS
**\$44 MILLION WITH CONCRETE
OR \$22 MILLION WITH STEEL.**
THAT'S THE DEFINITION OF A NO-BRAINER.

At least it was to the construction team on the Missouri River Bridge project when their initial concrete design priced over budget at almost \$45 million. They then had to scramble for other options. Turned out the solution was steel. After coming up with a new design, they turned to Nucor. And we were able to help them build a beautiful, easy to maintain and environmentally friendly bridge at less than half the cost of concrete. Who would've thought.

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BRITISH COLUMBIA AND QUEBEC 2009 STEEL DESIGN AWARDS



BRITISH COLUMBIA REGION AWARDS

ARCHITECTURAL AWARD

Award of Excellence

Robson Square Domes

ARCHITECT: Hughes Condon Marler Architects & Clive Grout Architects
STRUCTURAL ENGINEER: Read Jones Christoffersen Ltd.
OWNER: The Ministry of Labour's & Citizen's Services
GENERAL CONTRACTOR: Ledcor Constructors Special Projects
FABRICATOR: George Third & Son Ltd.
STEEL DETAILER: George Third & Son Ltd.
STEEL ERECTOR: KWH constructors

The Robson Square Sunken Plaza and Ice Rink Revitalization Project consisted of a new, extended ice skating rink, and two new "elliptical" stainless steel domes to replace the previous circular domes. In order to support the new domes, the existing supporting cantilevered concrete beams required substantial upgrading. Challenges were found in tightly fitting an "elliptical" dome into the existing "semi-circular" concrete supporting structure. The geometry of the dome is very intricate and required very precise detailing in order to have all the connections work. To make detailing and fabrication more efficient, the ring beam geometry was approximated by two different radii. Secondary tee ribs and tee on 6.625" diameter members were all straight and segmented. The 3-D model built using the X-Steel Program enabled precise detailing of all members for fabrication.

Each piece of the dome was preassembled in shop to ensure a good fit during installation at site. Because of the sheer size and weight of each dome, it was spliced into 4 sections before being shipped to site. The total weight of stainless steel for the project was 144,000 lbs. Once at site, each dome was successfully erected within 8 hours. Final fitting, welding and polishing needed to be completed on time in order to allow for the glazier to drop the glass panels onto the stainless steel dome.

ARCHITECTURAL AWARD

Award of Merit

Central Valley Greenway Bridge

ARCHITECT: Patkau Architects Inc.
STRUCTURAL ENGINEER: Delcan
OWNER: City of Burnaby
GENERAL CONTRACTOR: Smith Bros. & Wilson (BC) Ltd.
FABRICATOR: George Third & Son Ltd.
STEEL DETAILER: International Steel Detailing
STEEL ERECTOR: KWH constructors

ENGINEERING AWARD

Award of Excellence

Richmond Speed Skating Oval

STRUCTURAL ENGINEER: Fast + Epp Engineers
ARCHITECT: Cannon Design
OWNER: City of Richmond
PROJECT DIRECTORS: MHPM Project managers
GENERAL CONTRACTOR: Dominion Fairmile Construction
FABRICATOR: George Third & Son Ltd.
STEEL DETAILER: Tru-Line Drafting
CONSTRUCTION ENGINEERING: Somerset Engineering
STEEL ERECTOR: KWH Constructors

The arches for the Olympic Oval are the longest spanning hybrid steel-wood arches in the world. Spanning 100 meters, the arches are formed in a hollow triangular shape 2.1 meters high. A 10mm thick steel blade strengthens each arch on the bottom edge and metaphorically references the speed skating function. On the top side, W150 steel beams and 25mm thick steel plates strengthen the arch. The steel beams rise off of the wood slabs at both ends to create a raised roof and cantilever overhangs. A special steel jig was fabricated in order to bend and warp the wood and steel components to the correct geometry. The arches were constructed in 25 meter segments and field spliced with bolts, welding, and epoxy glue. The unusual design and tight arch tolerances required significant in situ fine tuning of erection procedures. The arches were temporarily and permanently stabilized by longitudinally oriented rows of HSS 300 x 300 that are curved to match the roof shape and moment spliced over the arches to prevent lateral torsional buckling of the arches. Tall gable end walls and lateral bracing in the east-west direction consist of structural steel framing. Custom steel castings were designed and manufactured for the yellow cedar struts supporting the cantilever overhang. The building has often been cast as a design featuring pine beetle wood, however the key to spanning the enormous 100 meter span was the combination of steel and wood to create a stunning and striking architecture.

KRENTZ AWARD

Winner

Vancouver Convention Center

OWNER: PAVCO/VCEP Ltd.
ARCHITECTS: LMN Architects, Musson Cattell Mackey Partnership, DA Architects + Planners
ENGINEER: Glotman Simpson Consulting Engineers
GENERAL CONTRACTOR: PCL Constructors Westcoast Inc.



cisc icca Lifetime Achievement Award

presented to
Mr. William Baird P.Eng.

In recognition of a lifetime dedicated to the advancement and understanding of steel structures in BC and in Canada through his unparalleled devotion to expanding the knowledge of the steel industry for the benefit of society, the industry and associated professions.

Presented by the BC Regional Committee of the
Canadian Institute of Steel Construction
November 2009



Left to right - Robson Square Domes, Central Valley Greenway Bridge, Richmond Speed Skating Oval, Vancouver Convention Center, Lifetime Achievement, Canron Aisle 6 Re-use.

STEEL FABRICATOR & ERECTOR: Canron Western Constructors Ltd.
STEEL DETAILER: Dowco Consultants Ltd.

The key solution for planning the structure turned out to be steel trusses for the full depth of floor levels and in a number of cases, multiple floor levels together with long span floor framing and roof framing. Steel joists provided a key part of the puzzle carrying heavy loads over long spans and easily integrated with the truss framing of the support walls. 10 foot deep trusses over the ballroom create a span between key bearing points that run down through the building onto the marine deck and onto the steel pipe piled foundations. Being able to span the distances necessary to reach key bearing points provided overall efficiencies that only the joists could provide.

Atop the sloping and irregularly shaped roofs is a living green roof, the largest of its kind in North America to date. Only with the use of steel roof joists could the structure have been as economical and quick to construct. With joists over the ballroom spanning to 120ft, the challenge of supporting the green roof was solved. Floors with a combination of composite beam and joist provided an efficient solution where vibration sensitivity was high and the depth available to mechanical and structure was very limited. The choice of a joist system helped the project maintain its budget and erection time-lines. Careful selection of systems helped contribute to the targeted LEED Gold Status.

LIFETIME ACHIEVEMENT

Winner

Mr. William Baird, P.Eng.,

The Lifetime Achievement Award was presented to Mr. William (Bill) Baird, P.Eng., in recognition of a lifetime dedicated to the advancement and understanding of steel structures in BC and in Canada.

SUSTAINABILITY AWARD

Award of Excellence

Canron Aisle 6 Re-use

OWNER: Canron Western Constructors Ltd. (A Supreme Group Company)
ENGINEER: Janto Engineering
DETAILER, ERECTOR: Supreme Steel Ltd.

HISTORY In 1974, Canron added a pre-engineered building "Aisle 6" to its Vancouver plant for fabricating bridges and large structures. After Canron moved out of its False Creek facility, the main plant was torn down in 1998, with the exception of Aisle 6—which stood for a further 15 years before being dismantled.

THE 3 R's – REDUCE, REUSE, RECYCLE In 2006, Aisle 6 was dismantled and shipped to Edmonton to be reused at the new Acheson plant. The main structural members as well as the original crane girders were reused and subsequently blasted, inspected and repainted. Both the two 20 Ton hoists and existing 20 Ton crane, along with the end trucks, were dismantled, refurbished, inspected and reused.

OLD BECOMES NEW The relocation of Canron's Aisle 6 to Edmonton has been a success for the Supreme Group of Companies. Costs were kept to a minimum because we were able to reuse an existing structure. Another advantage of reusing Aisle 6 was not having to design the Acheson plant from the ground up. This saved a significant amount of time for the company. It also reflects Canron's and Supreme Group's continuous commitment to sustainability in steel construction. The Acheson plant demonstrates how versatile and durable steel can be and how something old can be given new life.

QUEBEC REGION DESIGN AWARDS

COMMERCIAL / INSTITUTIONAL PROJECTS

Winner

Félix Leclerc Library Expansion, Quebec City

Jury Mention: For a balanced expression of the space including both the rigour and a harmonious integration between steel, wood and glass.

ARCHITECT: Anne Carrier Architect

STRUCTURAL ENGINEER: EMS Engineering

GENERAL CONTRACTOR: Constructions Pierre Blouin

OWNER: Ville de Québec

STRUCTURE MANUFACTURER: Quirion Métal Inc.

In the new section of the Val-Belar Library, in Quebec City steel is visible on the inside just as much as the outside. The steel structure of the existing building, built in 1987, had to come up to the standards, particularly seismic. A ductile steel plate shear wall has been incorporated. This technique was used for the first time in Quebec City. Since the beginning of the project, it was intended to give an architectural expression to the structure. The veil of structural wood is supported by the steel beams which fill up the space and then disappear before the large bay windows that look onto the park. All the glass walls also contain a steel structure which holds the mullions. The elegance of the steel and the thinness of the wood decking give lightness to the roof. On the back, an oculus was pierced above a masterpiece.



Left to right - Lebourgneuf Complex, Rapid Replacement of the Island Park Overpass Bridge, Nelson Mandela Bay Stadium, South Africa, Yankee Stadium,

COMMERCIAL / INSTITUTIONAL PROJECTS

Honourable Mention

Gatineau Sports Center

Jury Mention: For clarity of the concept adapted to the space and the landscape where steel meets concept criteria simply and efficiently.

ARCHITECT: Lapointe Magne et Associés

STRUCTURAL ENGINEER: Nicolet Chartrand Knoll Itée

GENERAL CONTRACTOR: Decarel inc.

OWNER: Ville de Gatineau

STRUCTURE MANUFACTURER: Structures Yamaska inc., Poutrelles Delta inc.

COMMERCIAL / INSTITUTIONAL PROJECTS

Honourable Mention

Lebourgneuf Complex, Québec City

Jury Mention: For the audacious integration of four ductile steel plate shear walls and their independent incorporation into the particular architectural design of the building.

ARCHITECT: Moreau & Beaudoin Architects

STRUCTURAL ENGINEER: EMS Engineering

GENERAL CONTRACTOR: Construction Richard Arsenault

OWNER: Lebourgneuf Complex inc.

STRUCTURE MANUFACTURER: Sturo Métal inc., Canam Canada

INDUSTRIAL PROJECTS / BRIDGES

Winner

Rapid Replacement of the Island Park Overpass Bridge, Ottawa

Jury Mention: For an extraordinary action strategy using the innovative prefabrication of steel structure and a rapid erection which reduced greenhouse gases emissions by 97%.

STRUCTURAL ENGINEER: McCormick Rankin

GENERAL CONTRACTOR: Dufferin Construction

OWNER: Ontario Ministry of Transportation

STRUCTURE MANUFACTURER AND DETAILER: Structal-ponts, div. Groupe Canam

INSTALLER: Montage d'Acier International inc.

TRANSPORTER: Mammoet Canada Eastern Ltd

To proceed with replacement of two twin bridges built with concrete slab on steel beams in 1959 on Highway 417 in Ottawa, an innovative solution was applied for the first time in Canada. The existing structures were removed and replaced using "self-propelled modular transporters" (SPMT). An adjacent site was used as a precast area for the two new structures. For the first time in North America, semi-integral abutments would be used. The use of steel shims

was required to ensure that 100% of girder load is carried by the bearing devices. According to the Ontario Ministry of Transportation, the use of this extraordinary technique in bridge reconstruction resulted in the cost savings of \$2.4 M compared with the standard, the work duration was reduced from 2 years to less than 17 hours. It also led to a 97% greenhouse gas emission reduction.

NON-QUEBEC PROJECTS

Winner

Nelson Mandela Bay Stadium, South Africa

Jury Mention: For its modern and elegant structural design, whose finesse of details makes the most of steel values.

ARCHITECT: GMP Architect Structural

ENGINEER: Schlaich Bergermann and Partners

GENERAL CONTRACTOR: Birdair Inc.

OWNER: Nelson Mandela Bay Municipality

DETAILER: Dessins Cadmax

Five new stadiums had to be built in South Africa in anticipation of the 2010 Soccer World Cup, the Nelson Mandela Bay Stadium in Port Elizabeth was one of them. This stadium is characterized by its spectacular location next to North End Lake. Its roof is a unique design which is no less spectacular than its location. It is made of Teflon-coated fibreglass and supported by 36 steel frames of cantilever trusses. Due to its flexibility, resistance and overall cost, this "airy" roof with both green and futuristic accents required the use of about 2500 tons of steel. The roof's construction has presented a major challenge, mainly due to geometric complexity of the structure. As many as 4200 assembly drawings had been required for the structure's erection.

Honourable Mention

Yankee Stadium, New York, NY

Jury Mention: For successful handling of various technical challenges related to an outstanding steel structure for an irregular and complex concept.

ARCHITECT: HOK Sport

STRUCTURAL ENGINEER: Thornton Tomasetti

GENERAL CONTRACTOR: Turner Construction

OWNER: New York Yankees

STRUCTURE MANUFACTURER AND DETAILER: Structal-construction

métallique lourde, div. Groupe Canam Structure

DETAILER: Dessins Cadmax



Garage Legendre and STM Collision Centre, Yobé-Trépanier Residence, Lionel Daunais Public Market

Honourable Mention

Fenway Park, repairs 2009, Boston, MA

Jury Mention: For the complexity of renovation work highlighting values of steel that contributes to the heritage preservation where each detail in itself is a technical and architectural challenge.

ARCHITECT: D'Agostino Izzo Quirk Architects, Inc.

STRUCTURAL ENGINEER: McNamara / Salvia Inc.

GENERAL CONTRACTOR: William A. Berry & Son, Inc

OWNER: Club de Baseball des Red Sox de Boston

STRUCTURE MANUFACTURER: Supermatal Structures inc.

DETAILER ENTREPRENEUR: Techdess inc.

GREEN BUILDINGS

Winner

Garage Legendre and STM Collision Centre, Montréal

Jury Mention: For its potential in reducing of the urban heat islands and the integration of a lasting, efficient and aesthetic steel structure.

ARCHITECT: Provencher Roy + Associés architects

STRUCTURAL ENGINEER: Pasquin St-Jean ingénieurs-conseils

GENERAL CONTRACTOR: Pomerleau inc.

OWNER: Société de transport de Montréal

STRUCTURE MANUFACTURER: Quirion Métal inc., Canam Canada

DETAILER: Genifab inc.

For the Garage Legendre expansion and new Collision Centre of the Montréal Transport Society, the priority has been given to noble materials of Quebec origin with low emissions and recycled content. An ecologically-based approach has been chosen: water recycling, plant walls, 950 square metres of green roof, 500 square metres of solar wall, rainwater retention pond with plants. The exterior of this MTS flagship project is composed of lath textured panels. Steel has been chosen for the structure due to the long clear spans according to the traffic lanes distribution needed to accommodate the new fleet of MTS articulated buses, in addition to the rapid structure erection.

YOUNG ARCHITECTS/ENGINEERS

Winner

Patrick Laurin – Aedicul, Montreal Airport, Dorval

Jury Mention: For its care about details and a successful steel integration in a complex set of architectural constraints and construction tolerances.

JUNIOR ENGINEER: Patrick Laurin, SNC-Lavalin

SPONSORS: Raymond Bleau, SNC-Lavalin; Alain Déom,

Nicolet Chartrand Knoll Itée Structure

MANUFACTURER: Michel Ménard, Structures Yamaska

RESIDENTIAL PROJECTS/RENOVATIONS

Winner

Yobé-Trépanier Residence, Chelsea

Jury Mention: For the audacious expression of a cantilevered steel structure and an artful expression of the steel beams apparent in a residential context.

ARCHITECT: Croft Pelletier Architectes

STRUCTURAL ENGINEER: EMS Ingénierie

GENERAL CONTRACTOR: Construction Pierbel

OWNER: Valérie Yobé et Marc Trépanier

Settled on a Chelsea hill in front of the Gatineau River, the new family residence of 225 m² was designed on three floors to minimize its land use. The two significant cantilevers created by opposite shiftings of the first and third levels reinforce the distinction between the three components, corresponding to as many functions (work, public life, private life). Magnificently integrated, the steel structure is present in every room, including the bedrooms. Even the central staircase is composed of a steel structure, structural wood decking and lumbers. This material was also chosen because of its architectural qualities, since the intermediate level is wrapped by corrugated steel and panels. The lightness of this level, as opposed to the bottom mass of concrete and the volume of superimposed wood, is entirely due to this unusual use of steel in the residential sector.

JURORS' FAVOURITE

Honourable Mention

Lionel Daunais Public Market, Boucherville

Jury Mention: For its modern language, the lightness of structure, the elegance of the rigid steel frame and apparent column hinges.

ARCHITECT: Sylvie Perrault Architectes

STRUCTURAL ENGINEER: Sylvain Parr et Associés

GENERAL CONTRACTOR: Les entreprises Claude Chagnon inc.

OWNER: Ville de Boucherville

NEWS AND EVENTS

WAIWARD STEEL FABRICATORS LTD. ACKNOWLEDGED AS ONE OF CANADA'S 50 BEST MANAGED COMPANIES

Waiward Steel has received recognition as one of Canada's Best Managed Companies. The firm first received this award in 2004.

Established in 1993, Canada's 50 Best Managed Companies is the country's leading business awards program, recognizing excellence in Canadian owned and managed companies with revenues over \$10 million. Every year, hundreds of entrepreneurial companies compete for this designation in a rigorous and independent process that evaluates the calibre of their management abilities and practices. The program sponsors are Deloitte, CIBC Commercial Banking, National Post, and Queen's School of Business.

For more information please go to the web site.

www.canadas50best.com

One more year of obtaining this recognition and Waiward will become a member of the program's Platinum Club, a rare, well-deserved honour.

Congratulations to all!



CISC AND SSEF ANNUAL CONVENTION

The CISC and SSEF Annual Convention will be held from June 9th to 12th, 2010 in the scenic Kananaskis area of Alberta! We look forward to an exciting time for all of our members throughout the 4-day convention. This year's convention is filled with educational seminars, marketing discussions and several wonderful and

informative speaker seminars, providing opportunities for you to learn the latest in steel industry ideas, trends and research initiatives during daily business and marketing meetings.

Also, this will be the first year where we celebrate the CISC National Steel Design Awards, where Regional Steel Design Award recipients will be judged and recognized on a National Level. Overall, a great evening that offers all attendees the opportunity to network with old colleagues, while making new contacts.

CISC ONTARIO REGION WILL EXHIBIT AT THE INTERNATIONAL BRIDGE CONFERENCE

The Ontario Region will be among the exhibitors at the 8th International Conference on Short & Medium Span Bridges, August 3 – 6, 2010. They will be located in booth 19. As you enter the exhibition hall, the booth will be down the right aisle – beside our colleagues from AISC. If you are attending the conference, do stop by and say hello! The conference will take place at the Sheraton on the Falls Hotel, 5875 Falls Avenue, Niagara Falls, ON.



NEW PUBLICATIONS

Handbook of Steel Construction, 10th Edition 2010

The 10th Edition of the CISC Handbook of Steel Construction will be released in June 2010! It is intended to be used in conjunction with the National Building Code of Canada 2010. Parts 1 and 2 feature the new CSA Standard S16-09 "Design of Steel Structures" and the CISC

Commentary, respectively. Design tables for eccentric loads on weld groups in Part 3 have been revised to incorporate recent research. Parts 4 and 5 contain information on compression and flexural members. In Part 6, section properties and dimensions data are provided for currently produced rolled and welded shapes. The CISC Code of Standard Practice, 7th Edition, leads the information found in Part 7 and features new guidelines on architecturally exposed structural steel and digital modelling.



Calcul des charpentes d'acier, Tome II

Beaulieu, Picard, Tremblay, Grondin, Massicotte
(Published in French)

This new volume is the second in a series of three books on the design of steel structures, with the first being published in 2003. Intended especially for graduate students, this second volume also constitutes a unique reference for practicing structural engineers. Practical design is emphasized throughout the book, which is illustrated with many numerical examples. Calculation formulas are based mainly on recent Canadian codes and standards, although the text often refers also to relevant international standards and scientific publications. The book consists of five chapters: *Steel and Its Properties – Supplementary Topics, Composite Beams, Plate Girders, Connection Design – Supplementary Topics, Brittle Fracture and Fatigue*.

INFORMATION RESOURCE FOR ARCHITECTS, ENGINEERS AND OTHER PROFESSIONALS

The Project Analysis Division (PAD)

As the national organization representing the structural steel, open-web steel joist and platework fabricating industries, the Canadian Institute of Steel Construction promotes safe, efficient and sustainable use of steel in construction. The Institute's Project Analysis Division, recognized as the Canadian source for information pertaining to design, costing and construction of steel structures, has routinely assisted structural consultants and other professionals for over four decades.

Whether you are in the search for the most suitable solution for a building or a bridge structure, do not hesitate to contact the Project Analysis Division for industry assistance.

Alfred Wong and his staff in PAD can be reached at:

Canadian Institute of Steel Construction

3760, 14th Avenue, Suite 200, Markham, ON L3R 3T7

Tel: 905.946.0864 x 110 | Fax: 905.946.8574

afwong@cisc-icca.ca

CONTINUING EDUCATION COURSES

Industrial Building Design

The course illustrates the limit states design of a single-storey industrial building. It refers extensively to the National Building Code of Canada 2005 (NBC 2005) and to CAN/CSA S16.1-05 "Limit States Design of Steel Structures" including the S16.1-05 Supplement, with emphasis on the applicability to typical Industrial buildings. In addition, there are references to the CISC Crane-Supporting Steel Structures: Design Guide, 2nd Edition and various AISC publications.

The example industrial building comprises common structural steel components used in roof and wall framing, such as roof trusses, crane runway beams, segmented columns, wall systems and standing seam roof systems. The building also serves to illustrate the design of a steel braced frame to resist wind and seismic loads, in accordance with NBC 2005 and S16-01. The course examines various design and construction topics, including; loads and load combinations, companion action approach, notional loads, vibration and fatigue, diaphragms, connections, foundations, coatings and corrosion considerations, low temperature toughness, rehabilitation, fire considerations and construction issues.

Toronto, ON	September 21, 2010
Edmonton, AB	September 22, 2010
Vancouver, BC	September 23, 2010

Bolting and Welding For Design Engineers

This popular course is designed to provide an introduction to the basics of bolting and welding of steel structures with emphasis on practical and economical solutions. Although not a connection design course per se, participants will come away with a solid understanding of the materials, products, specifications, installation, field challenges and design methodologies for connecting structural steel components.

Thunder Bay, ON	October 5, 2010
Toronto, ON	October 6, 2010
Saskatoon, SK	October 7, 2010
Calgary, AB	October 19, 2010
Kelowna, BC	October 20, 2010
Victoria, BC	October 21, 2010

Continuing education courses in development include:

- Inside the Steel handbook – Content and use of the new 10th Edition of the CISC Handbook of Steel Construction
- Practical aspects of steel construction and simple connections
- Design of connections in steel seismic force resisting systems
- Steel Bridges Design, Fabrication and Construction – Update to CSA S6-10 standard, plus a general refresh of the course material and a strengthening of fatigue and fracture topics

Please check the web site for updates: www.cisc-icca.ca/courses

CODES & STANDARDS REVIEW

Common Codes and Standards for Design and Construction of Steel Structures
Current Status and Future Publication Targets

code/standard/supplement/commentary	current edition	next edition/revision	publication target
National Building Code of Canada (NBC)	NBC 2005	NBC 2010	late 2010
NBC Structural Commentaries (Part 4 of Div. B)	NBC 2005 Str. Comm.	NBC 2010 Str. Comm.	late 2011
CSA S16 Design of Steel Structures	CSA S16-09	CSA S16-14	2014
CISC Commentary on CSA S16 (Part 2 of CISC Handbook of Steel Construction ¹)	CISC Handbook 10th Edition ¹ (2010/05/01)	CISC Handbook 10th Edition - 2nd Printing	2011
CSA S6 Canadian Highway Bridge Design Code	CAN/CSA S6-08	CSA S6-14	2014
- Supplements to CSA S6	CSA S6-06S1 (2010)		
		CSA S6-06S2	2011
CSA S6.1 Commentary on Canadian Highway Bridge Design Code	CSA S6.1-06	CSA S6.1-14	2014
- Supplements to CSA S6.1	CSA S6.1-06S1 (2010)		
		CSA S6.1-06S2	2011
CSA G40.20/G40.21 General Requirements for Rolled or Welded Structural Quality Steel/Structural Quality Steel	G40.20-04/G40.21-04 (R2009) ²	TBA	TBA
CSA W59 Welded Steel Construction (Metal Arc Welding)	CSA W59-03 (R2008) ³	CSA W59-11	2011
CSA W47.1 Certification of Companies for Fusion Welding of Steel	CSA W47.1-09	CSA W47.1-14	2014
CSA S136 North American Specification for the Design of Cold-Formed Steel Structural Members	CAN/CSA S136-07	CSA S136-15	2015
CSA S136.1 Commentary on CSA S136	CSA S136.1-07	CSA S136.1-15	2015

¹CISC Handbook of Steel Construction - 10th edition includes CSA S16-09, its Commentary, CISC Code of Standard Practice - 7th Edition, and design and detailing aids in accordance with CSA S16-09. ²Reaffirmed in 2009. ³Reaffirmed in 2008.

NEW MEMBERS

At the March meeting the CISC Board of Directors elected the following organizations as new members. Welcome all!

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www.jvdriver.com

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www.consortech.com

EVENTS

2010 CISC / SSEF Annual Convention

June 9 – 12, 2010 Kananaskis, AB

4th International Conference on Steel and Composite Structures

July 21 – 23, 2010 Sydney, Australia | www.iceaustralia.com

2010 Earthquake Conference

July 25 - 29, 2010, Toronto, Ontario | www.eeri.org

8th International Conference on Short & Medium Span Bridges

August 3 – 6, 2010 Niagara Falls, ON
www.bridgeconference2010.com

Steel Day

September 24, 2010

Various locations across Canada | www.steelday.ca

The Pacific Structural Steel Conference 2010

October 19 – 22, 2010 Beijing, China | www.pssc2010.com

International Symposium on Tubular Structures

December 15 – 17, 2010 Hong Kong, China
www.hku.hk/civil/ISTS13

Legend: *sales office only B-buildings Br-bridges S-structural P-platework J-open-web steel joist

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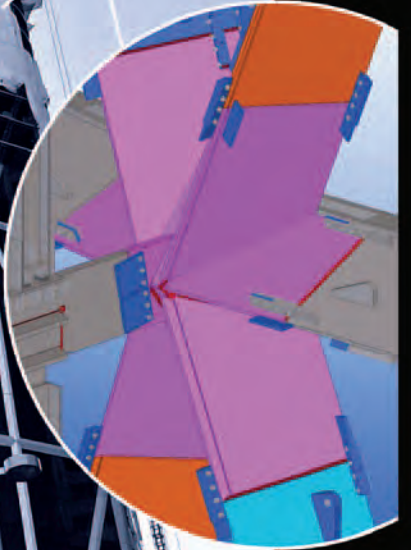
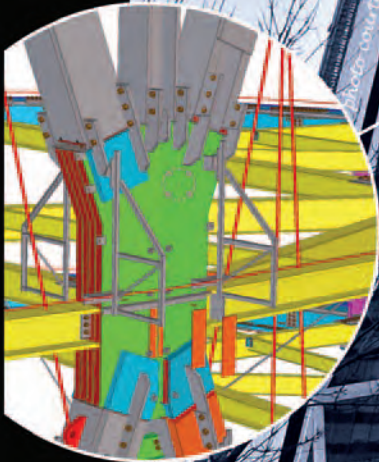
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