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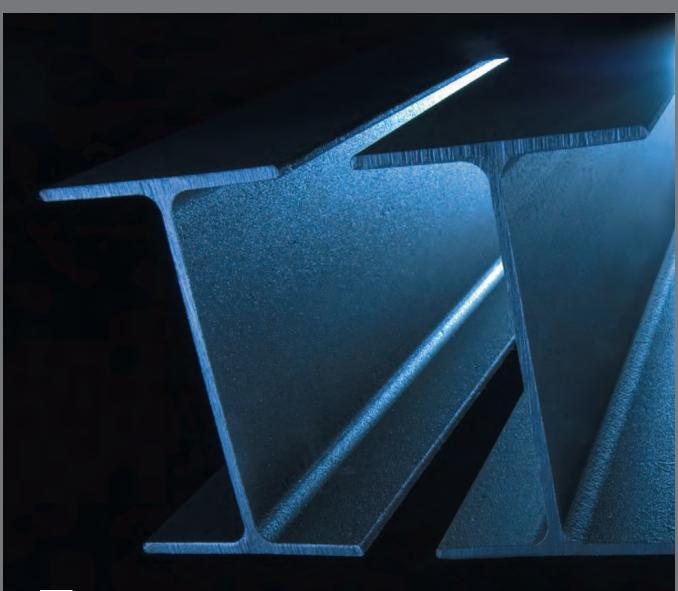




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FROM THE EDITOR

e did not set out to make this a HSS Special Issue however in many ways that is what has happened. Steel tubes are central to most of the articles. Not surprising from an architectural perspective. Hollow Structural Steel has long been used to provide beauty, environmental stewartship, as

well as function. We are now seeing more development and research work going into non standard shapes, pushing the architectural envelope, providing even more options for steel designers.

Professor Jeffrey Packer's fascinating series on Elliptical Hollow Sections continues with Part Two on EHS member design. He explores the technical specifications and engineering properties necessitated in the design of EHS. Carlos de Oliveira looks at ingenious solutions to the need for seismic-resistant concentrically braced frames using standardized brace connectors. While Seismic Corner launches Part 1 on buckling-restraining braced frames and one of the questions Ask Dr. Sylvie deals with is HSS corner radius. Ask Dr. Sylvie also deals with AESS categories, shortened outstanding angle legs and advises the 10th Edition of the CISC Handbook will be released by June 2010.

Also, we have an article examining three CISC Design Award winning structures launched as part of Quebec City's 400th anniversary. The image above of the EHS in the Telus Atrium in B. C. also received a CISC Design Award. Always an interesting read, our annual summary of scholarship and awards wraps up this issue.

My best wishes to you all, for a healthy and prosperous 2010! Ed Whalen, P.Eng. President CISC

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Professional engineers, architects, structural steel fabricators and others interested in steel construction are invited to enquire about CISC membership. Readers are encouraged to submit their interesting steel construction projects for consideration for inclusion in this publication by contacting CISC.

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Sylvie Boulanger, P.Eng. Ph.D. - Ask Dr. Sylvie is a column for Advantage Steel aimed at readers seeking technical information on steel structures. Questions are welcome on all aspects of design and construction of steel buildings and bridges. Suggested solutions may not necessarily apply to a particular structure or application, and are not intended to replace the expertise of a professional engineer, architect or other licensed professional. Questions for Dr. Sylvie, or comments on previous questions, may be submitted by e-mail to sboulanger@cisc-icca.ca.

ASK DR. SYLVIE

AESS CATEGORIES

We are bidding on an interesting project where the engineer has specified AESS2 and AESS3 in the specifications for the exposed steel portion of the job. Can you provide information on what additional work these categories represent to us in terms of fabrication? — R.T.

It's starting! As you recall, CISC has produced documents to help designers specify Architecturally Exposed Structural Steel (AESS). A full article was presented in the Summer 2008 issue (no. 31). The link to this article and all relevant AESS information is available on our website: www.cisc-icca.ca/aess

In terms of the expected work you are required to perform (and bid on) to meet the specifications for these Categories, you need to refer to the newest edition of the CISC Code of Standard Practice Appendix I – Architecturally Exposed Structural Steel, which became available earlier in 2009. Additionally, I imagine that the engineer has referred to the AESS Category Matrix in his or her specifications under the Structural Steel Division and includes requirements that resemble the Sample Specification provided at the same website address mentioned above. You should also note that the Ontario Region has developed a Sample Structural Steel Division 5 Specification document to ensure the most cost and schedule efficient project for the owner. That document is available from the Technical Resources of our website: www.cisc-icca.ca/ONDiv5Specs

I notice that for your project, the engineer specified AESS2 and AESS3: likely AESS2 for the portion that is over 6 m from view and AESS3 for the steel that's within closer view. That is very much how we expected the Categories would function i.e. one Category for a portion or group of members of the expressed structure. We imagine that architects decide in conjunction with the structural engineers, which Categories best suit the needs of the project. After that, the Categories appear on all design documents. For more visual examples, you can download a presentation that Terri Meyer Boake, Walter Koppelaar and myself gave at the Steel Conference: www.cisc-icca.ca/docs/aess/NASCC08 E15AESS.pdf

Finally, a Guide for specifying AESS is in preparation by Terri Meyer Boake, architecture professor at the University of Waterloo to help architects and engineers understand the AESS documents and more specifically the different characteristics that are associated to each Category. There will be loads of beautiful images to nurture your inspiration and appreciate what is more or less costly. There will be comments about galvanizing, paints and intumescent coatings. Our hope is to improve communication between architects, engineers, fabricators, detailers, erectors and suppliers so pricing and expectations are neatly aligned! We also secretly

hope that these documents will help reduce the amount of UUSS or Unfortunately Unexposed Structural Steel! Express the beauty of steel to satisfy fit, form and function.

HSS CORNER RADIUS

I am presently detailing a connection and I would like to know what the outside radius of curvature for a HSS 102x102x13 is? Actually, is there a fast rule for all HSS radii? — F.P.

The truth is the outside corner radius of a HSS varies with sizes and with each producer! Nonetheless, Table 13 of CSA-G40.20-04 defines the maximum exterior corner radius as a function of the wall thickness of the tube and the perimeter. For a rectangular tube with a nominal wall thickness of 13 mm and a perimeter smaller than 700 mm, the maximum outside corner radius presented in this table is 36 mm.

You should also note that to calculate the section properties in the Handbook, we use a value for the outside corner radius equal to twice the wall thickness and an internal corner radius equal to the thickness (see page 6-96 of the Handbook 9^{th} Edition). For HSS produced to CSA G40.20-04/G40.21-04 this thickness is taken to be the nominal wall thickness. For HSS produced to ASTM A500-07 this thickness is taken to be the "design wall thickness", or 90% of the nominal wall thickness (see page 6-97 of the Handbook 9^{th} Edition).

Finally, Professor Jeff Packer, of the University of Toronto, has measured outside corner radii on several recent HSS samples. Depending on the sample, the radii varied between 1.92 and 2.43 times the actual wall thickness measured. The entire article can be obtained from the AISC "Engineering Journal" First Quarter, 2005, or a summary found in the Fall 2003 issue of the Canadian Welding Association's journal at this address: www.cwa-acs.org/members/journal/Fall2003/Packer.pdf

SHORTENED OUTSTANDING ANGLE LEGS

You once wrote that for practical and economical reasons, some fabricators sometimes shorten the outstanding legs of a double-angle shear connection to fit a small HSS column. Has that been verified? — M.R.

Yes it has and the findings seem to indicate those connections have sufficient capacity. The concern was whether or not there was sufficient rotational capacity because of the reduced lever arm. You may recall that I was somewhat surprised to hear from an engineer that he often saw these connections proposed by fabricators. Upon doing a brief survey, it turns out that in some areas, this practice is common: fabricators either shorten the



outstanding legs and use fillet welds or they use flare bevel groove welds with full-section angles when the wall width of the HSS column is narrow (152 mm or less). So a research project was proposed and Yanglin Gong of Lakehead University was awarded a grant from the Steel Structures Education Foundation to perform the research work. The results confirmed that although there is less rotational capacity, it appears that all twelve full-size connections were able to undergo the required rotation. In other words, the connections reached their theoretical shear failure strengths without any premature failure, though shortened outstanding legs did reduce the rotational capacity.

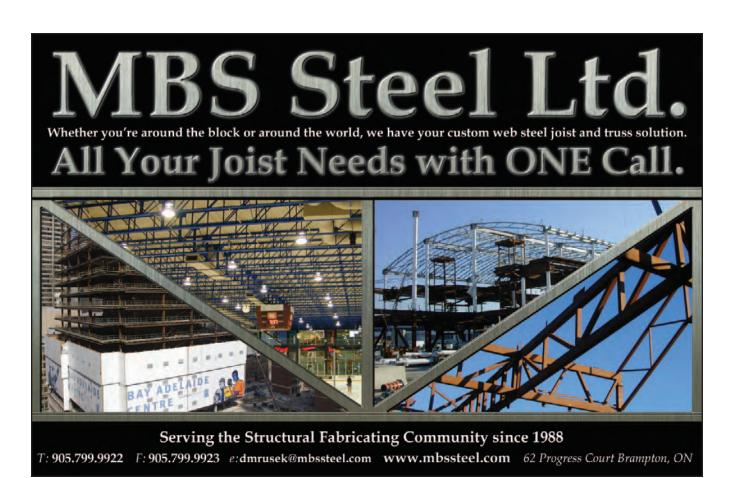
More specifically, the research results indicate that the practice of either shortening the outstanding legs or using flare bevel groove welds are both acceptable for double-angle shear connections with (1) outstanding legs not shorter than 30 mm; (2) angle thickness

from 6.4 mm to 9.5 mm; (3) the number of bolts not greater than 6, and (4) twice weld size top return for fillet welds and no top return for flare bevel groove weld. For more details, see the summary page on the SSEF website or read the CSCE journal article: www.ssef-ffca.ca/research/gong

NEXT HANDBOOK

I heard CSA-S16-09 was available for purchase and I wondered when the next Handbook Edition was going to be printed? — P.L.

Yes, \$16-09 can be ordered from CSA but it is not yet adopted by any of the provinces. However, we are planning to print the 10th Edition of the Handbook by June 2010. It will again contain the Standard in Part 1 and the Commentary in Part 2.



SEISMIC CORNER

BUCKLING-RESTRAINED BRACED FRAMES - PART 1

Alfred F. Wong, P.Eng.

YOU CAN HAVE STEEL BRACES THAT NEVER BUCKLE! ARE THEY TOO GOOD TO BE TRUE? READ ON...

Steel braced frames are essentially vertical trusses whose members are subjected primarily to axial forces only. As Eiffel demonstrated, they make the most efficient lateral-force resisting system. However, due to the strength of steel, relatively light and slender braces are used in many applications. The compressive resistance, C_r , for a slender brace is substantially smaller than its tensile resistance, T_r (For CSA G40.21 350W steel, T_r for a brace at the maximum permissible slenderness limit, T_r T_r

TRADITIONAL BRACED FRAMES

Moderately ductile and limited-ductility concentrically braced frames, as defined in CSA Standard S16 are proportioned to dissipate energy through inelastic action of the braces. Once the compression-acting braces have buckled and cannot regain their pre-buckling compressive resistance their tension-acting counterparts resist the bulk of the seismic forces. The above-mentioned reserve of tensile capacity due to brace slenderness is then mobilized provided the connections and other capacity protected elements can accommodate the redistributed forces in the post-buckling condition. A balanced frame configuration ensures sufficient tension-acting braces when the structure is subjected to motions in both forward and reverse directions.

BUCKLING-RESTRAINED BRACES

A buckling-restrained brace is prevented from buckling to allow the attainment of yielding in compression and strain-hardening. The resulting increase in compressive capacity usually permits the use of significantly smaller braces. A buckling-restrained brace





consists of a steel core and a buckling-restraining system that prevents the core from buckling, such that the brace's compressive resistance equals or exceeds its tensile resistance. Typically, the brace core is a flat steel bar that is confined in a buckling-restraining system which usually consists of a steel tube casing filled with mortar. Figure 1 shows the flat bar core of a brace and Figure 2 shows the view of the brace core inserted in its steel tube casing. The exposed ends are stiffened to preclude buckling. Their cruciform cross-section also facilitates field-bolted connection.

BUCKLING-RESTRAINED BRACED FRAMES

Physical tests have demonstrated that a well-proportioned buckling-restrained braced frame behaves very well when cyclically loaded well into the inelastic range. Figure 3 shows its typical stable, full and balanced hysteretic behaviour. Because of its system ductility CSA S16-09 recognizes the use of a ductility-related force modification factor, $R_{\rm dr}=4.0$. Moreover, the use of smaller braces leads to smaller capacity design forces and smaller connection design forces. Reduction in capacity design forces may also result in smaller beams and columns and reduction in foundation and diaphragm forces.

The use of buckling-restrained braced frames in hundreds of buildings in Japan and dozens in the U.S. has been reported in the literature. The IKEA Superstore in Coquitlam, B.C. that was completed in 2002, a CISC Steel Design Awards winner, is the first application of the buckling-restrained bracing system in western Canada (*Figure 4*). Comprehensive provisions for the design and testing of ductile buckling-restrained braced frames are introduced in the recently published CSA Standard \$16-09.

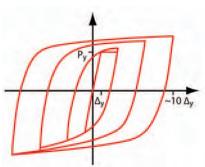


Figure 3 – Typical hysteresis of a buckling-restrained brace (Schematic)



Figure 4 – IKEA Superstore in Coquitlam, B.C. (A CISC Steel Design Awards winner in 2002)



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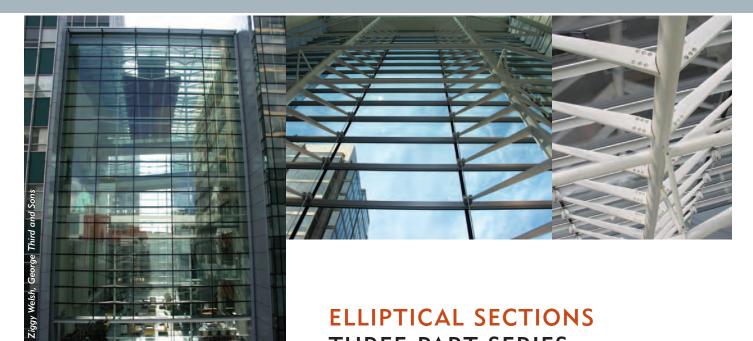












ELLIPTICAL SECTIONS

THREE-PART SERIES PART TWO: EHS MEMBER DESIGN

Jeffrey A. Packer

n Part One of this series, properties and applications of a new steel section on the market were presented. There is no doubt that many of our readers were excited to see these unique Elliptical Hollow Sections (EHS), perhaps for the first time. With a few Canadian applications already visible – as shown above with the Telus Atrium – architects are likely going to want to use them more often. Engineers will surely share their enthusiasm once they know how they can design them. Where can one find the properties and dimensions of EHS? How does one determine their slenderness? To what extent is their resistance in compression and bending different to Rectangular or Circular Hollow Sections (RHS and CHS)? How does one connect them?

Telus Atrium, British Columbia

The design of compression and flexural members requires knowledge of the cross-section classification. However, this is not covered by any current code, specification or standard at present. Fortunately, a Table of 46 shapes has been developed at the University of Toronto to provide engineering properties of EHS cross-sections. A sample Table is shown in this article. (The full list is available on the CISC website.) The format is very similar to the familiar Handbook of Steel Construction Tables.

The EHS section classification and member design issue has been pursued intensely by Gardner and colleagues in the U.K. and elsewhere. On the basis of experimental and numerical (finite element (FE)) studies, Gardner and colleagues have classified EHS into Classes 1, 2, 3 and 4 (per Eurocode 3 (CEN 2005)) with limiting wall slenderness ratios for various aspect ratios. Their system for cross-section classification has covered all prime loading cases: axial compression, bending about both principal axes and

combined compression plus bending. The Eurocode 3 class limits for CHS (very similar to those in Canada (CSA 2009)) were shown to be applicable to EHS if the EHS was treated as a CHS with an equivalent diameter of D_e. Two definitions of D_e have been used in the literature, which are herein termed De old and D_{e. new}, defined by:

D _{e, old} =	2a (a / b) for axial compression and minor axis bending	(1)
D _{e, old} =	1.3a (a $/$ b) for major axis bending, with aspect ratios of 2:1	(2)
D _{e, new} =	2a (1 + f [a / b] - 1) for axial compression,	(3)
D _{e, new} =	0.8a (a /b) for major axis bending, with aspect ratios of 2:1	(4)
with f =	1 - 2.3 (t / 2a) ^{0.6}	(5)

where a is half the larger EHS dimension and b is half the smaller EHS dimension, as illustrated in the schematic. That figure also illustrates the diameter of equivalent CHS (D $_{\rm e,\ old}$ and D $_{\rm e,\ new}$) for axial loading on an EHS with dimensions of 2a x 2b and of medium wall slenderness (2a/t = 35). The equivalent diameters can be seen to be much larger than the major dimension (2a) of the EHS. Thus, by taking the less conservative of Gardner's two approaches for axial compression loading, an EHS cross-section could be deemed to be non-slender (not "Class 4") providing (interpreting Table 1 of CSA-S16-09):

De, new
$$/t \le 23\ 000\ /F_y$$
 or $2a\ (1 + f[a/b] - 1)/t \le 23\ 000\ /F_y$ (6)

If one, however, looks at the local buckling failure mode of a typical EHS stub column in compression (see laboratory image), it resembles plate buckling more than cylinder (shell) buckling.

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U.S. Address: M.P.O. Box 1526 Niagara Falls, N.Y. 14302 - 1526 Thus, it was deemed that an "equivalent RHS shape" might be a better transformation of the elliptical tube, for the purpose of cross-section classification. Packer and colleagues proposed an equivalent RHS depth ($D_{\rm e,\ RHS}$) equal to 2a, while the width of the equivalent RHS was determined using the condition of maintaining the same cross-sectional area. This equivalent RHS shape is also shown in the schematic. It was possible to demonstrated that the same cross-sectional area.

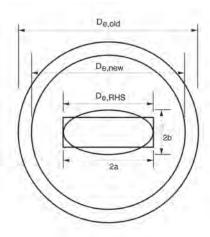


EHS stub column after failure, by inelastic local buckling or "squashing"; this mode of failure resembles plate buckling more than cylinder (shell) buckling.

strate that this "equivalent RHS" approach was a good estimator of whether EHS axially-compressed stub columns fail by yielding (squashing) or elastic local buckling, when compared with test results by Packer and colleagues as well as all others available internationally. In fact, the correlation with test results - using either the Eurocode 3 (CEN 2005) or CSA \$16 (CSA 2009) "Class 4" slenderness limit – was better for the "equivalent RHS" approach than for either of the "equivalent CHS" approaches. Thus, elastic local buckling was shown to be avoided if (interpreting Table 1 of CSA-S16-09):

$$(D_{e, RHS} - 4t) / t \le 670 / \sqrt{F_y}$$
 or $(2\alpha - 4t) / t \le 670 / \sqrt{F_y}$ (7)

The schematic, the laboratory results and the foregoing experimental validation for the axial compression loading case, suggest promise for the "equivalent RHS" method of handling elliptical cross-sections. Interestingly, if one applies the cross-section classification limits of equations (6) and (7) to the 46 shapes



Schematic of equivalent CHS diameters and equivalent RHS depth, for EHS in axial compression (for $\alpha/b=2$ and $2\alpha/t=35)$

available, one obtains almost identical consensus: by equation (6) 9 EHS are slender, and by equation (7) 8 EHS are slender, with the slender cross-sections generally occurring in large depth EHS.

The concept of applying the "equivalent RHS" method to define EHS members as Class 1, 2, 3 or 4 in *flexure* has been checked against available experimental data (18 tests by Chan and Gardner). For major axis bending the "equivalent RHS" approach is more accurate than the "equivalent CHS" approach. For minor axis bending (less common) the opposite is true, but the "equivalent RHS" approach is very conservative.

Hence, should engineers be faced with designing such a shape, two alternatives can be considered: the equivalent CHS or the equivalent RHS approach. At present, it appears that an equivalent rectangular shape shows the most promise for sizing a member. But what about connecting an EHS to another EHS? Current state of research will be presented in the next and third article on Elliptical Hollow Sections.

HOLLOW STRUCTURAL SECTIONS CSA G40.20 Elliptical

PROPERTIES AND DIMENSIONS

													'		
Designation	Wall Thick-	Mass	Dead Load	Area		Axis >	(-X			Axis	/-Y		Torsion Inertia Constant	Torsion Modulus Constant	Surface Area
	ness		Load		l _x	S _x	r _x	Z _x	ly	Sy	r _y	Z _y	J	Ct	As
mm x mm x mm	mm	kg/m	kN/m	mm²	10 ⁶ mm⁴	10 ³ mm ³	mm	10 ³ mm ³	10 ⁸ mm ⁴	10 ³ mm ³	mm	10 ³ mm ³	10 ⁸ mm ⁴	10° mm ⁸	m²/m
EHS 500x250 x12.5	12.50	112	1.10	14 200	350	1 400	157	1 960	118	943	91.0	1 200	353 000	2 110	1.21
EHS 320x160 x10	10.00	56.7	0.556	7 230	71.8	449	99.7	631	23.9	299	57.5	385	71 900	665	0.776
EHS 250x125 x10	10.00	43.8	0.429	5 580	33.2	265	77.1	376	10.9	174	44.2	228	32 900	385	0.606
EHS 200x100 x8	8.00	28.0	0.275	3 570	13.6	136	61.7	193	4.46	89.3	35.4	117	13 500	197	0.485

Sample Handbook Style Table of EHS Engineering Properties



The full list of References and the complete Table of Engineering Properties for the 46 EHS shapes are available on the CISC website: www.cisc-icca.ca/publications/ advantagesteel/36

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STEEL BY THE TONNE

FOR QUÉBEC CITY'S 400TH ANNIVERSARY!

Frédéric Simonnot

In Quebec, even though the facades are often made of stone, steel is the backbone of many structures, including some of the most famous, such as the Chateau Frontenac (yes, even that one!), the two bridges spanning the river or the Ile d'Orléans Bridge. This is even truer since the redevelopment of the airport, the restoration of the Espace 400° pavilion and the construction of the Brown Basin walkway, three projects produced shortly before the City's 400th anniversary celebrations and recognized by the CISC with awards in 2008.



he Québec City region today can take pride in a world-class airport infrastructure thanks to the major transformation of Jean-Lesage International Airport, just in time for the celebrations.

Previously, the air terminal configuration which was considered unwelcoming, resulted from a series of interventions carried out without any concern for architectural integration into the original building constructed about fifty years ago, resulting in difficulty reading the layout and poor organization of space.

This project necessitated demolition and reconstruction work on over 60% of the original surface. The mandate could be summarized as follows: improve the passenger service level and the functional aspects of the air terminal by better allocation of space; ensure sufficient operational flexibility to adapt easily to the users' and operator's requirements over a 20-year horizon; reduce the operating and maintenance costs; upgrade the general security level.

"The main challenges consisted of creating an international restructure that reflects the people of Québec City and that bears the signature of Québec engineering. This was achieved through a unique and distinct volumetry, without interrupting service, with a modest budget of about \$60 million," Serge Vézina summed up. He served as structural engineer and lead designer of the project for Dessau.

In accordance with the major principles of sustainable development, an effort was made to recover and integrate part of the existing

structures. Thus, the north sector of the air terminal was preserved, the new building was constructed in the footprint of the demolished part of the air terminal, part of the foundations, basement and structural slab of the ground floor was reused, and 90% of the demolished structure was sold. "The basement was preserved and the first concrete slab was superimposed on it. The metal frame was bolted to it with conventional anchors, buffers and shear keys for seismic purposes," Serge Vézina explains.

Outside, the existing ledge became an airside corrugated overhang, evoking an airplane wing and offering an easily identifiable visual signature. It then runs around the building, ending on the city side, where it joins the facade and supports a display screen. Inside, special attention was paid to the choice of materials in resonance with the Quebec identity and the development of vast, high and bright spaces offering a pleasant trip and precise control of thermal, visual and auditory comfort.

An economical, durable, easily implementable concept was chosen to meet the visual expression of the architectural volumetry and internal spatial organization, as well as the deadline imperatives.

The choice of steel was dictated by the big 16 to 18 m doors and the 13 m clearances required to create the glass canopies and give the roof its architectural signature. These canopies were produced with tubular steel sections maintained by a cable system, giving them the appearance of boat masts and allowing reversibility of suction and pressure stresses.



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"In this project, we tried to reconcile elegance and durability. This was a very interesting exercise for us, especially since each step had to be conceived to respect the requirement of maintaining airport activities," Serge Vézina recalls.

The roof support structures are conventional and exposed, except for the central span, which was designed and erected to allow passage of the main mechanical and electrical utilities hidden in the crawl space. The roof's lateral stability is assured by the combined action of rigid-hinged arches and horizontal braces made of steel, attached to the concrete shear walls built around the stairwells and elevator shafts.

The structure was manufactured and erected with the cooperation of all stakeholders, particularly the teaming of the manufacturer's

draftsman and the structural engineer. This cooperation was developed in virtual workshops, in which the 3D software used to produce the shop drawings was shared on Dessau's internet portal, a method that saved a lot of time. The CISC jury rightly presented an award to this project "for the clarity of the architectural concept, the aesthetic details of the fasteners and the sharing of the 3D model between the engineer and the manufacturer".

ARCHITECT: Consortium d'Architectes GPC

STRUCTURAL ENGINEER: Dessau inc.
GENERAL CONTRACTOR: Verreault inc.
OWNER: Aéroport de Québec inc.

STRUCTURE MANUFACTURER: Tecno Métal inc., Canam Canada



A SPACE AT THE HEART OF THE CELEBRATION

veryone who visited Québec City in 2008 remembers that the Espace 400e pavilion, on the edge of Louise Basin, was a flagship building of the celebrations, hosting many shows, workshops, debates, etc. It was enlarged considerably in anticipation of the celebration, the second biggest renovation of this old cement factory, already recycled for the "Québec 1984" event.

The building not only was enlarged and transformed. It was literally given a new skin! It is now enveloped in a double skin of steel and glass, a continuous, homogeneous and smooth curtain wall. This wall is a transparent showcase containing silk-screens of images chosen from the Port of Québec's photographic archives and allowing projection of images visible from both sides.

"Although the official application for certification was not made, this project sought to achieve LEED Silver classification criteria. This is one reason why we kept the envelope, except for the south part, which had to be deconstructed to proceed with the requested expansion," explains Gilles Prud'homme, an architect with Dan S. Hanganu architectes.

The CISC jury presented an award for this project, due to the unifying effect of the facade, its positive environmental aspects, its transparency and its lightness. It should be noted that it also received the Ordre des ingénieurs du Québec Award in the sustainable development category.

An airtight single glass curtain wall was thus suspended on a new steel structure fastened at two points to the concrete structure of the existing building. "The original use of the steel joists we fastened to the roof and the foundation wall allowed us to limit the fasteners and preserve the insulating properties of the existing envelope," affirms Marc Leblanc, structural engineer with SNC-Lavalin. This dynamic wall allows control of the captive air space between the new and old envelopes. Thanks to this "variable emissivity" envelope, the building changes appearance depending on the point of view, the time of day and the seasonal cycle.

A green roof, accessible for visitors, was developed on the west side, as well as a suspended walkway. A Trombe wall installed on the south facade acts as a solar radiation captor and accumulator. A geothermal system with radiant floors also contributes to heat the building.

"The real challenge for us was installation of vertical joists and coordination of the work with the other trades, which required very close monitoring," recalls André Goulet, President of Les Aciers Fax, the structure manufacturer.

"From the structural point of view, this project's great particularity was the use of steel joists as columns to support the glass walls, which is very rare. The main difficulty was to ensure their compatibility with the connections," confirms Sébastien Paré, structural engineer, in charge of inspection of the steel joists at Canam Canada.

The building now accommodates the Parks Canada Discovery Centre, which offers a new permanent exhibition on immigration, the St. Lawrence and the Parks Canada network.

ARCHITECT: Consortium Dan Hanganu + Côté Leahy Cardas

STRUCTURAL ENGINEER: SNC-Lavalin Inc.

GENERAL CONTRACTOR: EBC inc.

OWNER: Parks Canada / PWGSC

STRUCTURE MANUFACTURER: Les Aciers Fax inc.; Canam Canada



A WALKWAY OVER THE RIVER

he Brown Basin site is laden with history, because this is where General Wolfe's British soldiers landed in 1759. Subsequently, many shipyards and a railway pier were built there. Abandoned since 1973, it fell into obscurity. Since the federal government had decided to bequeath river sites to Québec City for its 400th anniversary, the Québec Port Authority wanted to redevelop the Brown Basin, a hope consistent with a movement to reclaim the river's shores.

The project stands out for the development of the site and the views it offers of the city and the river. Its development allows the creation of a visual and physical link between the shore and Québec's Upper Town, thanks to a pedestrian axis beginning at the foot of the Cap-Blanc stairway and ending on a jetty overhanging the river: a steel walkway.

"In fact, the walkway was imagined only in a second stage," confides Marc Letellier, an architect with Gagnon, Letellier, Cyr, Ricard, Mathieu Architectes. "Initially the aim was only to build the Brown Basin Interpretation Centre."

In a harmonious restoration, the roof of the centre in question today serves as a pedestrian link to the walkway. In the river, the anchoring and the foundation of the mast of this walkway are assured by one of the two existing mooring dolphin pillars. The other end is supported on the former marine infrastructures which must support a ship boarding ramp. The foundation work thus was minimized.

This is a catwalk guyed with three galvanized steel spans for a total length of 79 m and weighing 90 tonnes. It has a triangular tubular steel mesh structure. The height of the mast is 21.3 m (type A, rigid frame). The deck is treated yellow pine 86 mm thick.

"This is a special structure, in that the two angled masts give it the shape of a slightly tilted A, from which support cables were installed in one of the mooring dolphins. Having said this, the two main challenges will have been the size of the members and the tight deadline," Marc Letellier affirms.

The walkway's construction involved several technical challenges, due to the use of the existing foundations, the limited space, the complex structural geometry, and the implementation site's heavy exposure to wind and bad weather.

"This mandate required a lot more coordination than an ordinary project and we had to show ingenuity, because there was nothing in the Canadian Code on the type of assembly required to ensure the structure's strength and stability. We had to manufacture non-standard welded assemblies adapted to the context of the modules," recalls Jean-Louis Lemieux, a civil engineer with Sturo-Métal.

As can be imagined, transportation and installation were not easy either. "The walkway was made of five modules, which we had to convey by special transportation and install using two 220-tonne cranes, since the soil was too loose to support a more powerful machine. On the other hand, access to the site was difficult and it was never simple to work above the water," Mr. Lemieux remembers.

Reminiscent of a ship's architecture, the structure amplifies the axis created between the cliff and the river, especially at night when the pillars and all the bollards are illuminated. The CISC jury rewarded this project "for its aesthetic triangulation and success integration of the existing support members".

ARCHITECT: Gagnon, Letellier, Cyr, Ricard, Mathieu Architectes

STRUCTURAL ENGINEER: Tecsult | Aecom Inc.

GENERAL CONTRACTOR: Verreault inc.

OWNER: Québec Port Authority

STRUCTURE MANUFACTURER: Sturo Métal inc. DETAILER: Dessins de Structure DCA inc.

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AWARD-WINNING, CANADIAN-BORN TECHNOLOGY THAT SAVES DESIGN TIME AND SIMPLIFIES FABRICATION AND ERECTION

Carlos de Oliveria, M.A.Sc., P.Eng., Jeffrey A. Packer, Ph.D., P.Eng., Constantin Christopoulos, Ph.D., P.Eng.

SEISMIC-RESISTANT CONCENTRICALLY BRACED FRAMES

It is no secret that concentrically braced frames (CBF) are amongst the most popular lateral force resisting systems for medium to low-rise steel structures. This is mainly due to design and erection simplicity and to the increased stiffness that CBF provide in comparison to other lateral force resisting systems. In Canada, seismic-resistant CBF that are detailed for a ductile response come in two flavours: Moderately Ductile (Type MD) Concentrically Braced Frames and Braced Steel Frames with Limited Ductility (Type LD).

In the event of a design-level earthquake, Type MD and Type LD CBF dissipate seismic energy through the cyclic yielding and inelastic buckling of their brace members. It should be no surprise then, that the cross-sectional shape, cross-sectional slenderness, and overall slenderness of the brace members in both Type MD and Type LD frames determine the building's overall response in an earthquake.

In 2006, Packer suggested the use of round HSS or Pipe over rectangular HSS for energy-dissipating brace elements. New research by Fell et al (2009) supports this and suggests that wide-flange sections and round hollow section braces provide a more desirable seismic-resistant braced frame response than rectangular HSS braces. The authors point out that in these superior sections, local buckling occurs more gradually and thereby delays fracture initiation at the central plastic hinging point of the brace.

Practicing engineers can apply these principals to aid in the design of efficient ductile braced frames. It is commonly accepted that HSS are the most efficient structural shape for carrying compressive loading. Since bracing elements must be sized to carry compressive forces (excluding braces in tension-only systems), and given the aforementioned research findings, it makes sense to specify round HSS (produced to CAN/CSA G40.20/21 or ASTM A500) or Pipe (produced to ASTM A53) for the bracing elements in both Type MD and Type LD braced frames whenever possible. Once the compressive forces become too large to be carried by round HSS or Pipe elements (i.e. the axial compressive capacity of the available

sections that meet the stringent cross-sectional and overall member slenderness requirements for Type MD or Type LD frames is not sufficient), then wide-flange brace members should be specified.

These research findings, in addition to work previously done by Fell et al (2006) and as a result of recommendations made by Uriz et al (2007), are also behind the changes seen in the cross-sectional slenderness requirements for rectangular and round HSS seismic-resistant bracing in the newly published CSA steel design specification (2009) and the recent draft of the AISC seismic specification (2008).

The seismic response of CBF, characterized by cyclic yielding and buckling of their brace members, imparts arduous loading on the end connections of the activated braces. Consequently, CAN/CSA-S16 requires that all bracing connections in seismic-resistant CBF be detailed such that they are significantly stronger than the nominal cross-sectional capacity of the brace member. Specifically, for bracing elements in both Type MD and Type LD frames, the factored resistance of the brace connections must exceed both the probable tensile capacity of the bracing members in tension (given by $T_u = A_g R_y F_y$), and the probable compressive capacity of the bracing members in compression (C_u , given by the lesser of $A_g R_y F_y$ and $1.2 C_r/\varphi$ where C_r is computed using $R_y F_y$ and the probable post-buckling compressive resistance of bracing members). Detailing connections to provide this strength can be rather difficult, particularly when dealing with HSS.

SEISMIC-RESISTANT HSS BRACE CONNECTIONS

A slotted HSS-to-gusset connection is the most common detail used for connecting HSS brace members to the beam-column intersection. This type of connection induces shear lag in the hollow section, which can lead to connection facture at loads that are lower than the expected yield strength of the brace. Thus, it is good practice to provide net-section reinforcement in slotted hollow section bracing connections in ductile CBF. As discussed above, round HSS or Pipe elements make better energy absorbing bracing than rectangular HSS sections do, but the reinforcement of round sections requires the use of curved plate, channels, angles, or segments of other round sections, which can make detailing and fabricating the reinforced connection more onerous. Further com-



plicating the issue, the next edition of CAN/CSA-S16 will require the use of a probable yield stress, $R_y F_y$, of at least 460 MPa when designing HSS bracing connections, regardless of the specified minimum yield stress in the HSS being used. This change will significantly increase the size (and associated materials, labour and hence cost) of all HSS brace connections and brings our standard more in line with the AISC Seismic Provisions which have required the use of higher R_v values for HSS bracing for a number of years.

In terms of brace member fabrication, the slots that are cut or burned into the HSS itself must have smooth edges, as notches in the slots can become sites for crack initiation and propagation in the connection during an earthquake. Commonly, field welding of the fillet welds between the slotted HSS and gusset is specified, which can be costly and requires substantial quality control and field inspection. If field bolting is desired, the connections must be spliced as the load path must remain concentric, thus requiring a significant number of bolts, all of which must be pre-tensioned. In many cases, the number of bolts required for spliced, slip-critical brace end connections is prohibitive.

AWARD-WINNING CANADIAN INNOVATION – STANDARDIZED CONNECTORS

Recognizing the need for a simple solution to the seismic-resistant HSS brace connection dilemma, a research team at the University of Toronto headed by Professors Jeffrey Packer and Constantin Christopoulos developed standardized cast steel connectors shaped to eliminate the need for connection reinforcement. The connectors have made their way from the University laboratory to the field, and are now commercially available from the Toronto-based Cast Connex Corporation under their product line of High-Strength Connectors.

Although other industries such as rail, marine, mining, agriculture, energy and military make significant use of steel castings in structural applications, the North American steel construction industry has been slow to embrace the benefits of steel castings. The use of a steel casting to address the brace connection issues is a part of what made the Toronto research both innovative and practical.

At one end, High-Strength
Connectors are designed with a circular shape and preparation, which allows them to be shop welded to a round HSS brace.

The circumferential weld eliminates the occurrence of shear lag in the connection, and with a complete-joint penetration weld, the joint is inherently as strong as the brace itself, regardless of the actual overstrength of the HSS element. Further, the tapered preparation on the nose of the connector accommodates any HSS of a given outer diameter, regardless of the section's wall thickness – thus standardization of the connector is achieved. This standardization leverages casting manufacturing's inclination to mass production, making the connectors commercially practical.

On their other end, the connectors accommodate a double-shear bolted connection to the gusset plate. This bolt group can be detailed to provide a resistance commensurate with the probable tensile resistance of the particular HSS member being used. As the connectors are supplied without any bolt holes, the connection designer has the freedom to use whatever bolt pattern, grade, and diameter of bolt desired. Additionally, if slip-critical connections are warranted, the faying surfaces on the High-Strength Connectors are rough, reducing the number of bolts required as Class B slip-resistance can be assumed with the appropriate treatment of the gusset plate. Alternatively, if site fit up becomes an issue, or if the connections are to be exposed and the architect would like to avoid bolted connections, the connectors can be field welded to the gussets.

Having been developed in a university setting, braces equipped with High-Strength Connectors have been subjected to rigorous full-scale testing to prove their effectiveness in a design-level earthquake. This testing was carried out in the structures laboratories at both





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University of Toronto and École Polytechnique de Montréal and has been well documented (de Oliveira et al, 2008a, de Oliveira et al, 2008b, Tremblay et al, 2008).

The first project to feature braces equipped with these innovative connectors was a four-storey office building (expandable to six-storey) designed and constructed by CISC member Canam Group Inc. (Boucherville, Québec) and Les Architectes Odette Roy et Isabelle Jacques (St-Georges, Québec). The office building is to become the main administration building for Sandoz Canada Inc. and is part of a large new development which includes 40,000 square feet of manufacturing space, 10,500 square feet of warehousing space, and 17,000 square feet for administrative support. The site is located in Boucherville, Québec near the St-Lambert region of Montréal, which is a region of moderately high seismicity.

The Canam Group's Sandoz project, representing years of research and commercialization efforts, helped make the University of Toronto researchers and Cast Connex Corporation the inaugural winners of the Canadian Society for Civil Engineering's "Excellence in Innovation in Civil Engineering Award" in 2009. This prestigious award, presented by peers in the civil engineering community, recognizes outstanding innovation in civil engineering that has the potential for significant and far-ranging beneficial impact on the prosperity and well-being of society.



An additional design time-saving feature of the Cast ConneX® High-Strength Connectors: Cast Connex Corporation is about to launch a Design Manual that provides fully detailed double-shear bolted connections for every connector-to-brace combination possible for both bearing-type and slip-critical connections and for a variety of bolt diameters and grades. The manual also discusses everything from the full-scale product testing carried out by the company to a discussion on best practices for fitting, drilling, and welding to their connectors.

To pre-order your free digital copy, e-mail **HSCmanual@castconnex.com**.



For more information on Cast Connex Corporation, visit www.castconnex.com.

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SCHOLARSHIPS AND AWARDS SUMMARY

Rob White



G.J. JACKSON MEMORIAL FELLOWSHIP AWARD

The G. J. Jackson Fellowship is awarded annually by the Steel Structures Education Foundation in memory of the late Geoffrey Jackson. Mr. Jackson was for many years a leader in the

Canadian structural steel fabrication industry and was a founding member of the Steel Structures Education Foundation. The Award is presented to Canadian engineering students conducting graduate studies in structural engineering, with major emphasis on steel structures. This prestigious award is currently valued at \$15,000, over a one-year period. This award is presented at the SSEF Annual General Meeting.

The 2009 Jackson Fellowship recipient is **Hassan Moghimi**, from the University of Alberta. Hassan was presented with his award at the annual SSEF / CISC convention this past June in Winnipeg, Manitoba.

Hassan Moghimi is a Ph.D. student working under the supervision of R.G. Driver at the University of Alberta. Hassan's project is to investigate new applications of shear plate walls. Hassan's research will examine the effect of accidental blast on steel plate shear walls and the associated response and will also investigate the use of steel plate shear walls in low seismic zones. The aim of this research is to identify economic solutions for the use of steel plate shear walls in industrial structures.

2009 SSEF ARCHITECTURAL STUDENT DESIGN COMPETITION

"tension"

The Challenge

From an architectural perspective, the successful resolution of tension in a structure can be envisioned as the expression of an artistically satisfying equilibrium of opposing forces. While notions of tension might immediately bring to mind images of tensile structures, this exploration was not meant to be limiting in its scope. Students were invited not only to explore tension as it may be expressed in form, surfaces, and connections; they were also invited to engage in the exploration of tension as part of a structural dialogue that may occur between tension and compression as that results in the structural resolution of architectural form. While they may range from utilitarian to exquisite in their execution, all responses must, nonetheless, come to terms with one simple

problem: the clear application of tension to achieve a harmonious structural solution. To this end, the solution was not to hide this structural requirement; it must, instead, be celebrated and exploited, both architecturally and structurally.

Students were challenged to design a structure that explores "tension" on a site of the designers' choosing. While the purpose and scale are left to the discretion of the designer, it is important to focus on what it means for us to engage and experience structure as tension. The structure must be primarily steel, but otherwise, the material palette was open.

The jury consisted of Chris Adach (M & G Steel Ltd.), Neb Erakovic (Halcrow Yolles), Roger Pavan (Pavan Architects) and Carol Klleinfeldt (Kleinfeldt Mychajlowycz Architects Inc.).



Award of Excellence
Matt Schmid,
University of Toronto
Faculty Advisor: Philip Beesley
Amount: \$3,000

Award of Merit Jonathan Cummings, University of Toronto Faculty Advisor: David Bowick

Amount: \$2,000



2009 H.A. KRENZ RESEARCH AWARD

The H. A. Krentz award recognizes a researcher who's research topic has special merit and interest with promise that it will make a significant contribution to understanding the behaviour of steel structures, or advances in the economy, safe-

ty or reliability of steel structures. The 2009 H. A. Krentz Research Award is awarded to **Robert G. Driver**, **Ph.D.**, Department of Civil and Environmental Engineering, University of Alberta.

Professor Driver is the Chair of the Progressive Collapse Subcommittee (A279.6) developing the CSA Standard S850, "Design and Assessment of Buildings against Blast". He is also on the CSA S16 Committee and provides essential coordination between the two standards. His primary research interests pertain to the design and behaviour of steel structures, including topics such as bolted & welded connections, plate walls, high performance

steel, rehabilitation, composite columns, and behaviour under extreme loading.

The Steel Structures Education Foundation awarded a grant of \$24,000 for Professor Driver's research on the "Development of Canadian Progressive Collapse Mitigation Criteria for Steel Structures". In addition, a gift of \$5,000 is given as part of this notable award. Professor Driver also received this award in years 2006, 2007 and 2008.

2009 SSEF ARCHITECTURE SCHOLARSHIP

The SSEF is pleased to provide scholarships to students enrolled in accredited professional Schools of Architecture across Canada. These students must show innovation and excellence in steel design. The precise criteria for the award were developed by the individual School and Faculty / Administration.

LAVAL UNIVERSITY

For projects conceived during a workshop of design:

Alexandre Guilbeault – Bachelor's Level Guillaume Drouin Chartier – Master's Level Simon Pelletier – Master's Level

UNIVERSITY OF MONTREAL Award of Excellence

Jean François Marceau

Second Prize

Marc-Antoine Grondin Vincent Carrière-Marleau Simon Chawky

UNIVERSITY OF BRITISH COLUMBIA

Excellence in Steel Design

Charlotte Falk Kali Gordon Katy Mulla

CISC also offers a number of scholarship award programs and initiatives for students across Canada. Funded and administered through regional efforts, these initiatives are offered to students conducting studies in the field of structural engineering, and are designed to help promote structural steel studies at Canadian education institutes.

ATLANTIC REGION

The Atlantic region's scholarship program is open to applicants who will be doing a postgraduate degree on research in structural steel structures or a related topic at one of the four Atlantic Engineering Universities (University of New Brunswick, Université de Moncton, Dalhousie University and Memorial University). Two awards, each in the amount of \$2,500 are available annually.

One award has been presented in $2008\,/\,2009$ to Frankie Soloman of Dalhousie. His research work is directly related to better

characterize the behaviour of extended shear connections. This award is in recognition of his achievements to date and for his continuing advancement of structural steel studies at Dalhousie University.

The 2009 / 2010 Atlantic Region graduate scholarship program has been modified . Only one scholarship with a value of \$7500 will be offered starting in 2009 / 2010. The Atlantic Regional Committee of the Canadian Institute of Steel Construction (CISC) has established this graduate scholarship to support an engineer who is pursuing a post graduate degree in Civil Engineering with emphasis on structural steel structures or a related steel topic. The applicant can be a recent engineering graduate or an engineer that is working in industry, government or the academic field. The goal is to provide monetary support to a person that is continuing their study in the structural steel field while encouraging that person to continue their career in the steel industry.

ONTARIO REGION

The Ontario Regional Committee awarded scholarships in 2009 to students who excelled in their steel design courses, eight of which were presented to engineering students and two to architectural students. Chosen recipients were selected based on input from their professors at each respective institution. This year scholarships went to:



- ☐ **Bryan Boutilier**, Windsor University, sponsored by Benson Steel & Mirage Steel
- Matthew Charbonneau, Waterloo University, sponsored by M & G Steel & Walters Inc.
- Devon Comstock, University of Western Ontario, sponsored by Dymin Steel & CISC Ontario
- Daron Keskinian, University of Toronto, Engineering,
 sponsored by Telco Steel Works & Mariani Metal Fabricators
- Mark Reinders, McMaster University,
 sponsored by Walters Inc. & Telco Steel Works
- ☐ Mark Steenhof, Ryerson University, Engineering, sponsored by Skyhawk Steel & MBS Steel
- □ Juan Giraldo Velez, Carleton University, sponsored by Dymin Steel & M & G Steel
- University of Toronto, Architectural,
 sponsored by Mariani Metal Fabricators & CISC Ontario
- Queen's University,sponsored by Benson Steel & Mirage Steel
- ☐ **Ryerson University**, Architectural, sponsored by MBS Steel & Skyhawk Steel

These awards provide each recipient with \$2000 in scholarship funding. The applicants must be undergraduate students who excel in the steel design course during their third year and who also

selected a steel elective in their final year. The award presentations were part of the Ontario Region's 19th Annual Spring Reception held May 13, 2009 at the Living Arts Centre in Mississauga.

BRITISH COLUMBIA REGION

The BC Regional Committee has offered a Fabricator's Engineering Apprentice program for the past nine years. The program formally integrates a UBC student's academic studies with work experience in co-operative employer organizations, for a four-month work-term working with both a CISC fabricator and structural engineering consultant. Congratulations to the following students who were selected to participate in the 2009 program. The CISC steel fabricator employer is also listed. These students were presented with a certificate award at the BC

Region's 2009 Steel Design Awards of Excellence in Vancouver held on November 19, 2009.

- □ Stephen Clark, Canron Western Constructors Ltd.
- □ Osvaldo Mote, George Third & Son Ltd.

CENTRAL REGION

The Central Regional Committee has established an annual scholarship award in the amount of \$2,000, which is presented to a student(s) enrolled in the College of Engineering at the University of Saskatchewan.

For more information about these education initiatives or to find out how to apply for an award, please contact your regional director or visit the websites at www.cisc-icca.ca and www.ssef-ffca.ca.





NEWS AND EVENTS

CONGRATULATIONS TO FAST + EPP AND CANNON DESIGN

Fast + Epp Structural Engineers and architectural firm Cannon Design of Vancouver received the Institution of Structural Engineers' Award for Sports or Leisure Structures for the Richmond Olympic Oval. The award, which acknowledges achievement in structural design, was announced in London, England on Friday, October 9th at the Institution of Structural Engineers 2009 Structural Awards Gala and marks the first major international award for the Richmond Olympic Oval.

CONGRATULATIONS TO SUPERMETAL STRUCTURES ON THEIR 50^{TH} ANNIVERSARY

Since its founding in Quebec in 1959, Supermetal has made teamwork a priority. Their recipe for success depends on the motivation of each employee. From the design to the erection of structural steel, from computer drafting to the smallest little weld, every link in the chain is important. Each employee is called upon to add his best and work together with his colleagues to achieve a product and service that go beyond industry standards and clients' expectations. As in a symphony orchestra, each employee's outdoing himself is the ideal to strive for, hence the vital importance of teamwork.

Congratulations on your 50th and best wishes for the next 50!



CISC ONTARIO MEMBERS TOUR THE UNIVERSITY OF TORONTO'S STRUCTURAL TESTING FACILITY

On October 7, members of the Ontario Region were invited by Professor Christopoulos to visit the new structural testing laboratories at U of T. The labora-

tories have undergone major upgrades in order to acquire state-ofthe-art testing and sensing equipment that will be used to carry out extensive testing on steel structures.

Prior to touring the facility; several students gave presentations on recent research projects. These students were recipients of the G. J. Jackson Fellowship or funded by SSEF. During the tour, a test on elliptical steel tube connections was carried out by an Ontario Region Scholarship Recipient.



CISC ONTARIO ANNUAL GOLF DAY

The Ontario Region held its annual Golf Day on September 19th at the Carlisle Golf & Country Club. The winning four-

some, Neil McMillan, Tsvet Tsokov, Steven Law and Tim Haskett received the award for the "Lowest Score". Thanks to Mike Minielly for coordinating a fun-filled day enjoyed by member and guests.

Standing – left to right – Mike Minielly (Dymin Steel), Randy Abbott (Skyhawk Steel Ltd.), Neil McMillan (AECOM), Tsvet Tsokov (Telco Steel Works), Steven Law (Reinders + Rieder Ltd.), Tim Haskett (Canam Canada)

Seated – **left to right** – Tom Lau-Wiffin, CISC Ontario Chair (Ed Lau Ironworks), Paul Ast (Jablonsky, Ast and Partners)

TEACHING AID FOR THE WINNIPEG CAMPUS OF THE RED RIVER COLLEGE

CISC Central Region is fabricating a teaching aid for installation on the Red River College campus. The teaching aid is basically a column tree with a number of steel elements attached to the column using a variety of connection methods. Over the past number of years numerous such teaching aids have been placed on University and College campuses allowing students to see how actual structural steel elements are being used every day in the construction of buildings, bridges and other structures.

For more information of this program, contact your CISC Regional Manager.

SAINT JOHN HIGH SCHOOL SOFTWARE TRAINING UPDATE

Training of the high school teachers from the Saint John area who will be using the software SDS II in their technology programs, took place at Ocean Steel's training facility in June. The regional CISC representative gave a talk on the role of CISC in the steel industry with emphasis on the opportunities in the steel business. On September 2 - 3 the Saint John region technology high school teachers were at Ocean Steel for the remainder of their SDS II training. Alan Lock spent the morning of Sept $2^{\rm nd}$ presenting a comprehensive view of the structural steel industry and its importance in the Atlantic Provinces economy. That afternoon there was a guided tour of the PCS Piccadilly mine site.

2010 ASCE/AISC NATIONAL STUDENT STEEL BRIDGE COMPETITION

CISC and SSEF are proud sponsors of the ASCE/AISC National Student Steel Bridge Competition. The design of bridges is perhaps the most exciting challenge for a structural engineer. This competition fosters the challenge of designing and testing a bridge. Students are encouraged to apply their theoretical knowledge in a hands-on project that addresses the full breadth of steel design requirements, including: aesthetics, speed of erection, lightness, stiffness, economy and efficiency.

The 2010 competition will take place May 28 – May 29 at Purdue University, Indiana. AISC and the competition co-sponsors assist with travel funds for those teams invited to compete. The first top team from each region receives (US) \$1000. The second top team from each region receives (US) \$500. SSEF contributes \$1000 to each Canadian team that qualifies for the National competition. SSEF also tries to match a team with a local CISC Steel Fabricator. CISC Regional Committees provide varying levels of financial support for Canadian teams attending regional competitions. For more information, please go to the SSEF web site. www.ssef-ffca.ca

CONTINUING EDUCATION COURSES - SPRING 2010

All courses listed here will be held in the **Spring of 2010**. Specific dates for the locations are to be confirmed. Please check the web site for updates: www.cisc-icca.ca/courses

Industrial Building Design

The course illustrates the limit states design of a single-storey industrial building. It refers extensively to the National Building Code of Canada 2005 (NBC 2005) and to CAN/CSA-S16-01 "Limit States Design of Steel Structures" including the S16S1-05 Supplement, with emphasis on the applicability to typical Industrial buildings. In addition, there are references to the CISC Crane-Supporting Steel Structures: Design Guide, 2nd Edition and various AISC publications.

St. John's, NL Spring 2010 Moncton, NB Spring 2010 Ottawa, ON Spring 2010 Saskatoon, SK Spring 2010 Calgary, AB Spring 2010

Conception de bâtiments industriels

Ce cours illustre la conception des états limites d'un bâtiment industriel à un seul étage. Il renvoie fréquemment au Code national du bâtiment du Canada 2005 (CNBC 2005) et à la norme CAN/CSA-S16-01 « Règles de calcul aux états limites des charpentes en acier », supplément à la norme S16S1-05 compris, et plus particulièrement à l'applicabilité aux bâtiments industriels types. Le cours renvoie également au « Crane-Supporting Steel Structures: Design Guide, 2nd Edition » de l'ICCA ainsi qu'à diverses publications de l'AISC.

Québec, QC Printemps 2010 Trois-Rivières, QC Printemps 2010 Montréal, QC Printemps 2010

Bolting and Welding For Design Engineers

This popular course is designed to provide an introduction to the basics of bolting and welding of steel structures with emphasis on practical and economical solutions. Although not a connection design course per se, participants will come away with a solid understanding of the materials, products, specifications, installation, field challenges and design methodologies for connecting structural steel components.

St. John's, NL Spring 2010 Halifax, NS Spring 2010 Fredericton, NB Spring 2010 Ottawa, ON Spring 2010 Toronto, ON Spring 2010 Winnipeg, MB Spring 2010 Regina, SK Spring 2010 Edmonton, AB Spring 2010 Vancouver, BC Spring 2010

Boulonnage et soudage à l'intention des ingénieurs en structure Ce cours propose une introduction à l'assemblage des charpentes d'acier et insiste sur les solutions économiques et pratiques. Bien que

d'acier et insiste sur les solutions économiques et pratiques. Bien que ce ne soit pas un cours de conception d'assemblages proprement dit, les participants en tireront cependant une solide compréhension des matériaux, produits, caractéristiques, installations, problèmes sur le chantier et méthodes de calcul pour l'assemblage des charpentes d'acier.

Québec, QC Printemps 2010 Trois-Rivières, QC Printemps 2010 Montréal, QC Printemps 2010

Conception parasismique d'immeubles à charpente d'acier

Ce cours très en demande sera offert une nouvelle fois dans les centres à activité sismique du Canada et abordera la conception de diverses catégories de contreventements et cadres rigides selon les exigences du Code national du bâtiment du Canada (CNBC) – Édition 2005 et de la norme CSA \$16-01 (\$16\$1-05) en incorporant des exemples de conception d'immeubles allant de un à dix étages.

Quebéc, QC Printemps 2010 Montréal, QC Printemps 2010

NEW MEMBERS

FABRICATOR

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261, avenue du Havre, Rimouski QC G5M 0B3 Tel: 418 723 2610 Fax: 418 725 4485

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EVENTS

IABSE-fib Conference

May 3 – 5, 2010, Dubrovnik, Croatia www.iabse.org/conferences/Dubrovnik2010/index.php

NASCC - The Steel Conference

May 12 – 15, 2010 Orlando, U.S.A.

www.aisc.org

ASCE/AISC Student Steel Bridge Competition

May 28 – 29, 2010 Purdue University, U.S.A.

www.aisc.org/content.aspx?id=780

2010 CISC / SSEF Annual Convention

June 9 – 12, 2010 Kananaskis, AB

 4^{th} International Conference on Steel and Composite Structures July 21-23, 2010 Sydney, Australia

www.iceaustralia.com

8th International Conference on Short & Medium Span Bridges

August 3 – 6, 2010 Niagara Falls, ON www.bridgeconference2010.com

The Pacific Structural Steel Conference 2010

October 19 – 22, 2010 Beijing, China

www.pssc2010.com

International Symposium on Tubular Structures

December 15 – 17, 2010 Hong Kong, China

www.hku.hk/civil/ISTS13

REGIONAL ACTIVITIES

Alberta Region - "The Steel Workshop" March 25, 2010

Ontario Design Awards

May 13, 2010 - The Living Arts Centre, Mississauga

Legend: *sales office only B-buildings Br-bridges S-structural P-platework J-open-web steel joist

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www.atcongroup.com Cherubini Metal Works Limited	C D	Jean Yves Fortin Soudure Inc. Montmagny, QC	S 418 248 7904	Supermétal Structures Inc. St. Romuald, QC www.supermetal.com	S, P 418 834 1955
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Legend: *sales office only B-buildings Br-bridges S-structural P-platework J-open-web steel joist

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Legend: *sales office only B-buildings Br-bridges S-structural P-platework J-open-web steel joist

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Edmonton, AB www.behlen.ca Blastal Coatings Services Inc.	780 237 8497	Welded steel/ aluminum/stainless steel grating, ' "Shur Grip" safety grating	'Grip Span" and	Moore Brothers Transport Ltd. Brampton, ON www.moorebrothers.ca	905 840 9872
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www.bolair.ca Paint spray equipment & accessories, ie: horses, vo spray guns, etc.	lves, filters,	Frank's Sandblasting & Painting Nisku, AB	780 955 2633	Edmonton, AB www.pricesteel.com Pure Metal Galvanizing, Division of PMT	
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Brunswick Steel Winnipeg, MB www.brunswicksteel.com	204 224 1472	Globec Machineries / Globec Machinery Québec, QC www.globec-machinery.com	418 864 4446	Red River Galvanizing Inc. Winnipeg, MB www.redrivergalvanizing.com Supplier of hot dip galvanizing only	204 889 1861
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The Sherwin-Williams Company Ville d'Aujou, QC www.sherwin.com Specialty industrial coatings	514 356 1684	SSAB Enterprises, LLC Lisle, IL www.ssab.com	630 810 4800	Saskatoon, SK www.wilkinsonsteel.com Misc. structural shapes, hot rolled bars and plates. Structurals- angles, flats, beams, channel, plate	306 652 7151
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www.pipe-piling.com Hot Roll-Wide-Flange-Bearing Pile Beams VARSTEEL Ltd. [Delta] Delta, BC	604 946 2717	Acier Leroux Boucherville, Division de Métaux Russel Inc. Boucherville, QC www.leroux-steel.com	450 641 2280	Wilkinson Steel and Metals, A Division of Premetalco Inc. Cranbrook, BC www.wilkinsonsteel.com	250 489 3333
www.varsteel.ca Beam, angle, channel, HSS plate, Sheet, pipe, flats, rounds, etc.		Acier Pacifique Inc Laval, QC www.pacificsteel.ca	514 384 4690	Misc. structural shapes, hot rolled bars and plates. Structurals- angles, flats, beams, channel, plate Wilkinson Steel and Metals,	
VARSTEEL Ltd. [Lethbridge] Lethbridge, AB www.varsteel.ca Beam, angle, channel, HSS plate, Sheet,	403 320 1953 Gratina. expanded metal.	Dymin Steel (Western) Inc. Abbotsford, BC www.dymin-steel.com	604 852 9664	A Division of Premetalco Inc. Kamloops, BC www.wilkinsonsteel.com Misc. structural shapes, hot rolled bars and plates. Structurals angles, flats, beams, channel, plate	250 374 7122
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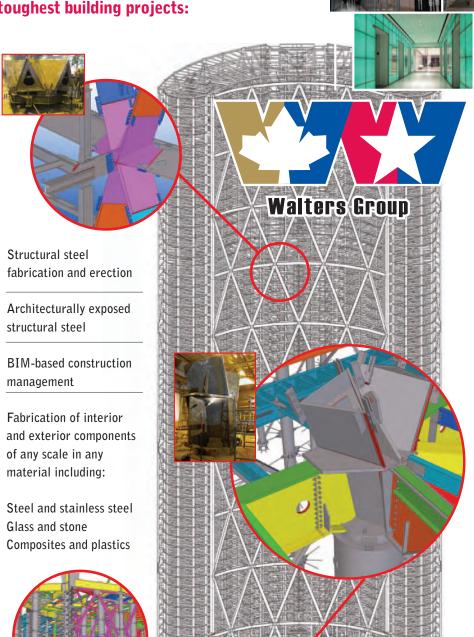




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